Herne Bay Phase 6A

Drainage Strategy Technical Note

October 2022

132592-FAH-ZZ-XX-TN-C-02-00











CONTROL SHEET

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REPORT TITLE: Drainage Strategy

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Revision Record	1			Check		
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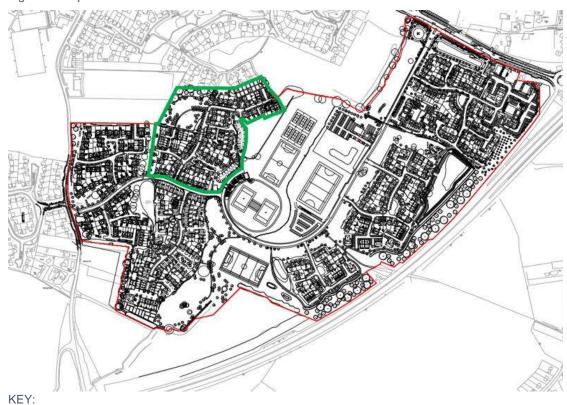
1 Introduction

1.1.1 Fairhurst has been commissioned by Redrow to prepare a Drainage Strategy in relation the proposed development taking place in the town of Herne. The strategy relates to planning consent CA/15/00844/OUT and the discharge of Condition 13 and 14 which relates to surface water drainage and foul sewerage.

1.2 Site Description

1.2.1 The site is 4.63ha in size and is currently greenfield. The topographic survey (Appendix A) indicates that the land generally falls towards the southeast of the site and rises from the north towards the middle and eventually falls towards the east and south. The highest area is in the west of the site where levels reach circa 18.73m AOD, the lowest levels are in the east of the site where levels reach a minimum of circa 9.34m AOD.

Figure 1: Proposed Site Plan



Phase 6 Boundary

Planning Boundary

Source: Redrow Homes

1.3 Planning History

Original Hybrid - CA/15/00844/OUT

1.3.1 A hybrid planning permission was granted by Canterbury City Council in September 2015 subject to conditions and a S106 legal agreement, for the redevelopment of the Herne Bay Golf Course. The full element of the consent comprised the construction of 113 dwellings (Phase 1A) in the northern part of the site, a central spine road and a central sports hub, comprising a sports pavilion, tennis pavilion together with football, cricket, hockey pitches and tennis courts.



- The outline element of the consent then included up to a further 459 dwellings, a 60 bed care home, a doctors surgery, public house and commercial uses.
- 1.3.2 The residential part of the redevelopment scheme is being built out by Redrow Homes, with the sports provision having been provided by Quinn Estates, who are also developing out the other remaining parcels outside the outline residential area.

Amended Hybrid Consent - CA/16/00378/VAR

- 1.3.3 The original hybrid consent was then amended on the 14th October 2016.
- 1.3.4 The S73 application made a number of amendments to the first phase of residential development, for which full planning permission had been granted, which included reducing the amount of units consented from 113 to 109 and making associated changes to the layout of the parcel.

Reserved Matters Approvals Phase 3 (also referred to as Phase 1B)

- 1.3.5 The following reserved matters approvals have subsequently been granted:
 - CA/16/02131 Phase 3 156 dwellings (20th December 2017)
 - CA/17/01296 Phase 4 102 dwellings (15th November 2018)
 - CA/18/02369 Phase 5 109 dwellings (30th August 2019)
 - CA/20/00101 Phase 6A 92 dwellings (1st July 2021)

Other Planning Permissions

1.3.6 A separate full planning permission was granted on the 8th November 2019 for four detached dwellings together with a public house and office building, under reference CA/18/02299. At the same time consent was also granted under reference CA/18/02290 for 93 dwellings, 1,179sqm of office space and 372sqm of retail space across various different blocks of development within the development. CA/20/01946 was subsequently granted in July 2021 for a foodstore and 8 flats, in place of office space originally approved on the site.

Other Planning Permissions

1.3.7 Planning application CA/22/00026 has been submitted by Quinn Estates and is currently being determined. Planning permission is being sought for 95 residential units across various blocks of development together with other non-residential uses. Six of those proposed units are within Block E, and would replace the two units previously consented in that block under 18/02290.

1.4 Scope of the Report

1.4.1 The report has been prepared to discharge Condition 13 and 14 of planning consent CA/15/00844/OUT in relation to the Phase 6A part of the site. The remaining residential plots, and design of the basins, wetlands and final outfall from the site would be dealt with separately.

Condition 13

No Development within a Construction Phase shall commence (save for any Advance Infrastructure and Enabling Works) until a surface water drainage based on sustainable drainage principles and an assessment of the hydrological and hydrogeological context of the development, has neem submitted to and approved in writing by the Local Planning Authority. The drainage strategy should demonstrate the surface water runoff generated up to and including the 100 year plus climate change (e.g 30% increase in intensity) critical storm will not exceed the runoff from the site discharging onto the highway. Also the strategy shall include details of the design, location and capacity of all such SuDS features and shall include, ownership, long-term management/maintenance and monitoring arrangements/responsibilities.

The scheme shall subsequently be implemented in accordance with the approved details before the development is completed.



Condition 14

No development within a Construction Phase shall commence (save for any Advance Infrastructure and Enabling Works) until details of the proposed means of foul sewerage disposal have been submitted to and approved by the Local Planning Authority, in consultation with Southern Water. The development shall not be carried out other than in accordance with the details as approved.

2 Proposed Drainage

2.1 Design Parameters

2.1.1 The proposed drainage strategy has been developed in line with national and local planning policy/guidance. Key design parameters used in the development of the strategy are summarised below:

Rainfall data: FEH 2013Total site area: 4.6ha

Proposed drained area: 1.7ha

Design storm: 1 in 100 year event plus climate change (40% increase in rainfall intensity)

 Surface water attenuation is downstream of the Phase 6A site has already been constructed.

2.2 Discharge Locations

Surface Water

2.2.1 Surface water runoff would be discharged in line with the drainage hierarchy set out in the PPG. This is set out within Table 1 below.

Table 1: Surface Water Drainage Hierarchy

Discharge Method	Suitability/Comment	
Infiltration to ground	British Geological Survey (BGS) records show that the bedrock geology of the area is London Clay which consists of clay and silt. Therefore, the ground conditions are unsuitable for discharge.	
To a surface water body	Surface water flows in Phase 6A would be discharged to the existing attenuation basin/swales that has already been constructed, which ultimately discharge to the Plenty Brook.	
To a surface water sewer	Not required.	
To a combined sewer	Not required.	

Foul Water

2.2.2 Foul flows would be discharged to the existing Southern Water foul network run underneath Randal Way Road located south of the site, which was previously constructed to serve the wider site.

2.3 Sustainable Drainage Systems

2.3.1 Sustainable Drainage Systems (SuDS) are drainage systems that are environmentally friendly as opposed to the conventional piped sewer system. They are designed to mimic natural



drainage, slow down runoff rates to reduce flooding and prove the quality of water entering watercourses. They also manage the amount of water entering watercourses or the ground at a controlled rate by means of storage or slow infiltration.

2.3.2 Table 2 below summarises the key SuDS features that are available as outlined in the CIRIA SuDS Manualⁱ.

Table 2: Sustainable Drainage Systems

SuDS Feature	Description	Suitability/Comment
Rainwater harvesting	Rainwater harvesting is the collection, storage, treatment (where necessary) of rainwater runoff from roofs and other impermeable areas for reuse within the site. In addition to reducing volume runoff from the site, they can reduce the water demand of the site delivering climate resilience and sustainability benefits.	Not proposed.
Green roofs	Green roofs are areas of living vegetation included on the roofscape of buildings. They can be either extensive or intensive and accessible or non-accessible. The plant and soil reduces the rate of discharge extending the time between rainwater falling on the roof and reaching the rainwater outlet / drain. They also provide ecological and visual benefits.	Green roofs are not suitable on the proposed pitched roofs.
Infiltration Systems	Infiltration systems hold water and allow it to percolate back into the ground as it would naturally in permeable areas. These can either be traditional shallow soakaways or deep bore soakaways. Their suitability depends on the soil permeability. Due to the effect of water on structural stability, these need to be sited sufficient distances from buildings / foundations. These can reduce volume runoff from sites and contribute to recharging groundwater	Infiltration systems are not suitable for the site due to the bedrock makeup being unsuitable for infiltration.
Proprietary treatment systems	Proprietary treatment systems are manufactured products to remove specified pollutants from runoff. These can reduce downstream maintenance requirements and provide additional benefit, if required, by receiving watercourses / discharge locations.	Proprietary treatment systems are not proposed for this site as sufficient treatment would be provided through the use of SuDS.
Filter strips	Filter strips are uniformly graded gently sloping strips of grass or vegetation to treat runoff by slowing down flows, promoting sedimentation and infiltration.	Filter strips are not proposed because there is not sufficient space on the plots.
Filter drains	Filter drains are shallow trenches filled with gravel to attenuate, treat and convey surface water runoff. They can convey /	Filter drains are not suitable due to



SuDS Feature	Description	Suitability/Comment
	attenuate only or, depending on site conditions, allow infiltration direct to the ground.	underlying ground conditions.
Swales	Swales are shallow flat bottomed channels to convey, infiltrate (where possible) and treat surface water runoff. They can enhance site design and provide biodiversity enhancements. They are often used to drain roads, paths or car parks. Swales can replace traditional pipes as a means to convey flows and used as part of a SuDS train of elements.	Swales are proposed for attenuation and conveyance of surface water runoff.
Bio-retention systems	Bio-retention systems including rain gardens are shallow landscaped depressions to treat and store runoff using engineered soils and vegetation. They provide amenity and visual benefit alongside additional climate benefits. They are usually used for containing / managing frequent storm events.	Rain gardens not proposed.
Trees	Trees help protect the environment in a number of ways including reducing runoff rates through interception of rain water in their canopies, and promoting infiltration in permeable / soft landscaping as well as the visual benefit they provide to the area.	Trees are proposed on the site.
Pervious pavements	Pervious pavements provide pavement surfaces suitable for pedestrian / trafficked applications whilst allowing runoff to permeate through their structure. This provides filtration benefit to treat runoff. Pervious pavements can be used to collect, treat and convey flow only, or if site condition permit, allow infiltration to the ground direct from their base.	Pervious pavements are not proposed.
Attenuation storage tanks	Attenuation storage tanks temporarily hold back water for gradual release or reuse at a controlled rate to reduce the peak runoff rate. These can be in the form of above ground tanks (blue roofs), below ground geocellular / concrete tanks or oversized pipes.	Below ground geocellular attenuation tanks are not proposed on site.
Detention basins	Detention basins are landscaped depressions which are normally dry except for during and immediately after storm events. These attenuate flows through controls on the outfalls to store rainwater upstream in networks providing treatment and amenity benefits. With careful design,	Detention basins are proposed for the site and have already been constructed.



SuDS Feature	Description	Suitability/Comment
	these can be used for leisure / amenity uses during normal / dry periods.	
Ponds & wetlands	These are similar to detention basins, however they are designed to have a permanent level of water within them to provide biodiversity and amenity benefits.	Ponds and wetlands are not proposed for the site.

2.4 Surface Water Drainage Strategy

- 2.4.1 The proposed drainage network has been designed to connect to an existing attenuation basin located to the east of the Phase 6A site. This basin was previously approved and constructed as part of an early phase of the site wide development.
- 2.4.2 The areas to the southwest, southeast and middle of the site discharge to the attenuation basin via two swales located in the east of the site (Appendix B), which have already been constructed. The surface water drainage run that serves the northern part of the site runs through the proposed Phase 6B site and discharges to the same approved and constructed basin east of the 6B site (Appendix B). The previously constructed and approved attenuation basin restricts flows in line with the consented strategy.
- 2.4.3 MicroDrainage Network calculations (Appendix C) have been undertaken to ensure that the drainage runs have been sized appropriately to contain the runoff flows generated by the site. The calculation accounts for all storm durations and takes account of a 40% increase in rainfall intensity to account for climate change.
- 2.4.4 The basin and swales, which are already constructed, would provide water quality, water quantity, biodiversity and amenity benefits.
- 2.4.5 The surface water strategy for Phase 6A is shown on the drawings included in Appendix B. Exceedance flow routes have also been indicated on this drawing.
- 2.4.6 The drainage strategy has been granted Technical Approval by Southern Water (Appendix D).

2.5 Water Quality

2.5.1 Appropriate treatment would be incorporated into the drainage system to ensure that the quality of water discharged is acceptable in line with the CIRIA SuDS Manual. This would be achieved through the incorporation swales and attenuation basins.

2.6 Maintenance and Management

- 2.6.1 The onsite drainage networks and SuDS would be privately managed and maintained for the lifetime of the development, to ensure they remain fit for purpose and function appropriately. The management company/operator would be appointed post-planning.
- 2.6.2 Table 3 overleaf, outlines what maintenance is anticipated for the proposed SuDS features.



Table 3: Sustainable Drainage System Maintenance – SuDS Manual C753

Maintenance Schedule	Required Action	Typical Frequency
	Detention Basins	
	Remove litter and debris	Monthly
	Cut the grass – for splillways and access routes	Monthly (during growing season), or as required
	Cut grass – meadow grass in and around the basin	Half yearly (spring, before nesting season, and autumn).
	Manage other vegetation and remove nuisance plants	Monthly (at start and then as required)
	Inspect inlets, outlets, banksides, structures, pipework, etc for evidence of blockage and/or physical damage	Monthly
Regular Maintenance	Inspect banksides, structures, pipework etc for evidence of physical damage.	Monthly
	Inspect inlets and facility surface for silt accumulation and establish appropriate removal frequencies.	Half yearly
	Check any mechanical devices, e.g penstocks	Half yearly
	Tidy all dead growth (scrub clearance) before start of growing season.	Annually
	Remove sediment from any inlets, outlet and forebay.	Annually (or as required)
	Manage wetland plants in the outlet pool – where provided	Annually
	Reseed areas of poor vegetation growth	As required
Occasional	Prune and trim any trees and remove cuttings	Every 2 years, or as required
maintenance	Remove sediment from inlets, outlets, forebay and main basin when required	Every 5 years, or as required (likely to be minimal requirements where effective upstream source control is provided).
Remedial actions	Repairs erosion or other damage by reseeding or re-turfing	As required



	Realignment of rip-rap	As required
	Repair/rehabilitation of inlets, outlets, and overflows	As required
	Relevel uneven surfaces and reinstate design levels.	As required
	Trees	
	Remove litter and debris	Monthly or as required
Regular maintenance	Manage other vegetation and remove nuisance plants	Monthly (at start, then as required)
	Inspect inlets and outlets	Inspect monthly
	Check tree health and manage tree appropriately	Annually
Occasional maintenance	Remove silt build-up from inlets and surface and replace as much as necessary	Annually or as required
	Water	As required (in periods of drought)
Monitoring	Inspect silt accumulation rates and establish appropriate removal frequencies	Half Yearly
	Swales	
	Remove litter and debris	Monthly
	Cut grass- to retain grass height within specified design range	Monthly (during growing season) or as required
	Manage other vegetation and remove nuisance plants.	Monthly at start, then as required
	Inspect inlets, outlets and overflows for blockages and clear if required	Monthly
Regular maintenance	Inspect infiltration surfaces for ponding, compaction, silt accumulation, record areas where water is ponding for >48 hours.	Monthly, or when required
	Inspect vegetation coverage	Monthly for 6 months, quarterly for 2 years then half yearly
	Inspect inlets and facility surface fir silt accumulation, stablish appropriate silt removal frequencies.	Half-yearly



Occasional Mantainance	Reseed areas of poor vegetation growth, alter plant types to better suit conditions if it requires	As required or if bare soil is exposed over 10% or more of the swale treatment area
	Relevel uneven surfaces and reinstate design levels	As required
Remedial actions	Scarify and spike soil layer to improve infiltration performance, break up silts deposits and prevent compaction of the soil surface.	As required
	Remove build up, sediment on upstream gravel trench, flow separator or at top of filter strip.	As required
	Remove and dispose of oils or petrol residues using safe and standard practices using safe standard practices.	As required

2.7 Foul Drainage Strategy

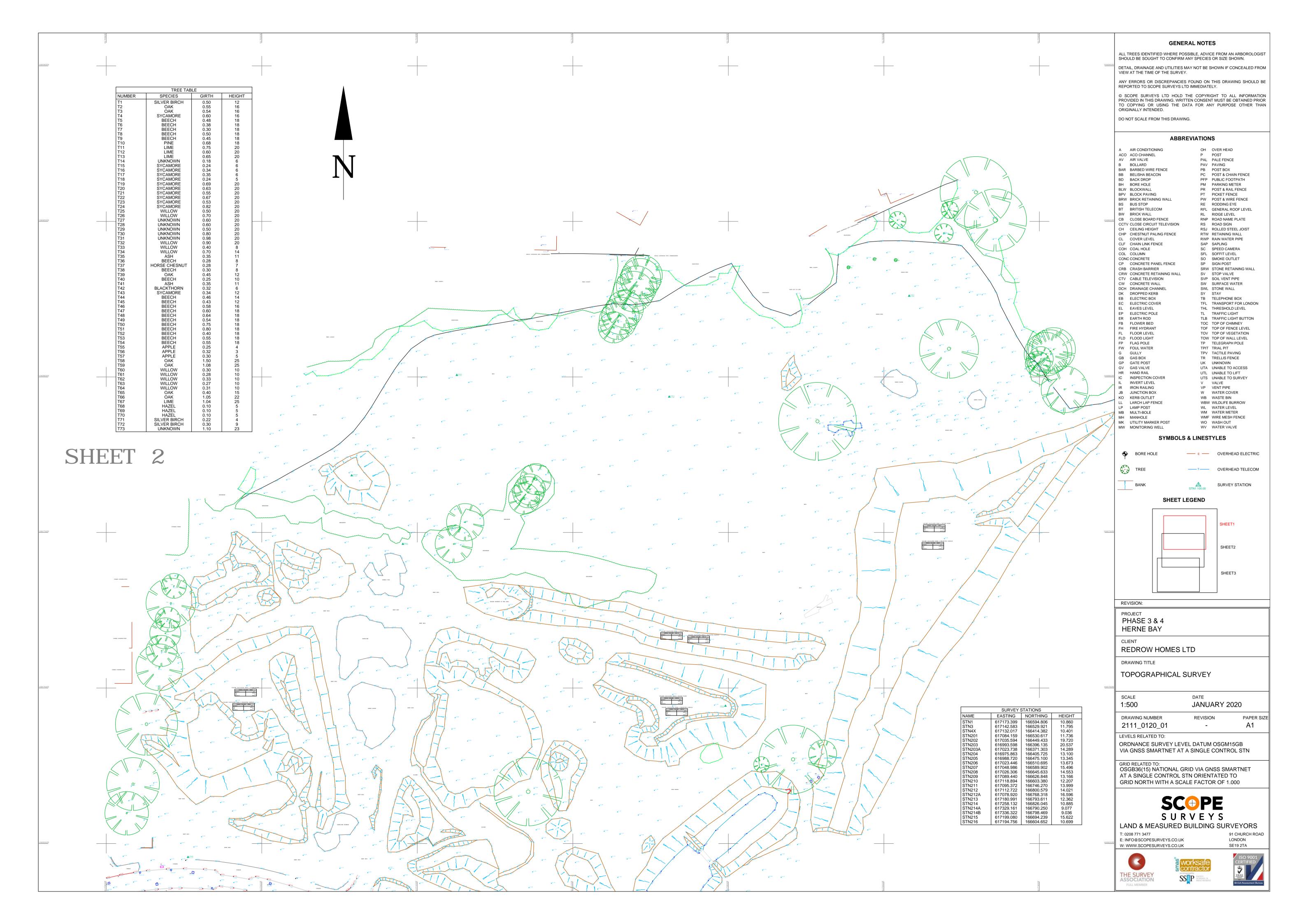
- 2.7.1 Foul drainage has also been accounted for and is designed to be discharged into the foul drainage run located underneath Randall Way Road. The foul drainage run located along Randall way will be ultimately discharged to the foul pump station located east of the site.
- 2.7.2 The foul water strategy for Phase 6A is shown on drawing 132592-FAH-ZZ-00-DR-D-0501-0504 MAIN DRAINAGE LAYOUT (Appendix B). Exceedance flow routes have also been indicated on this drawing.

3 References

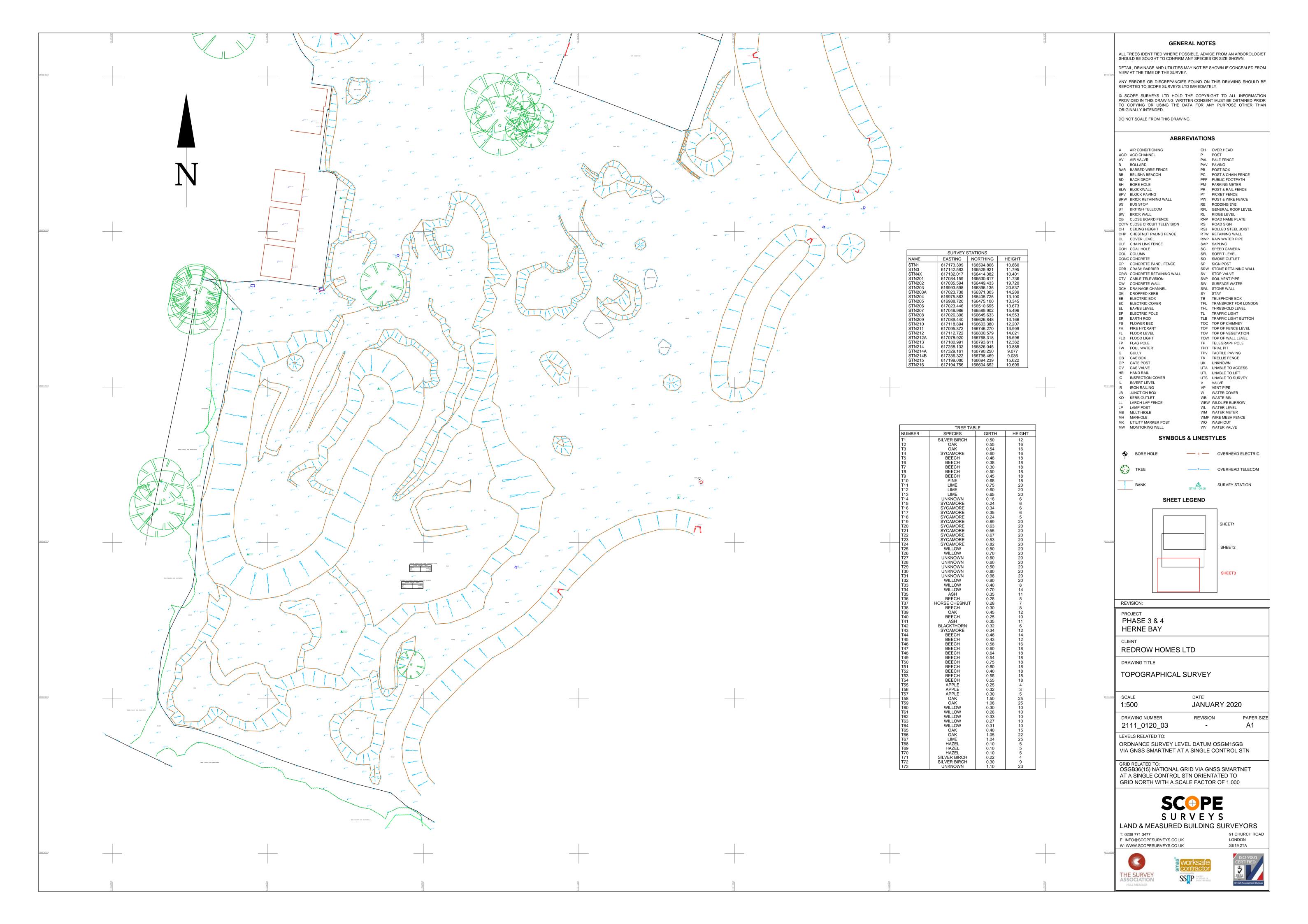
¹ CIRIA C753, 2015. The SuDS Manual.



A. Topographical Survey





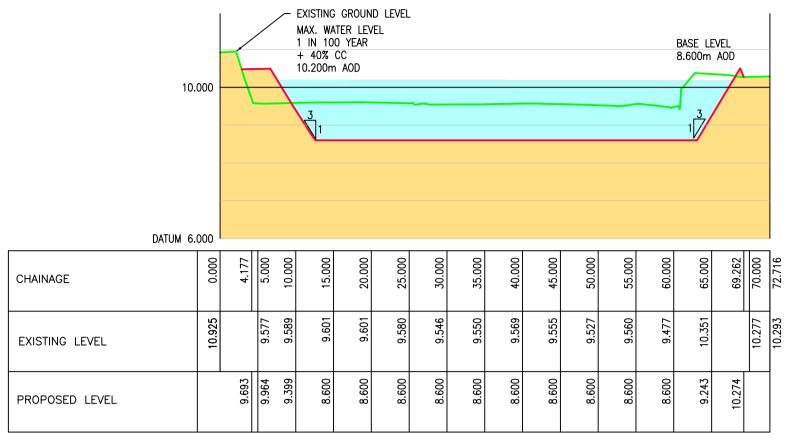


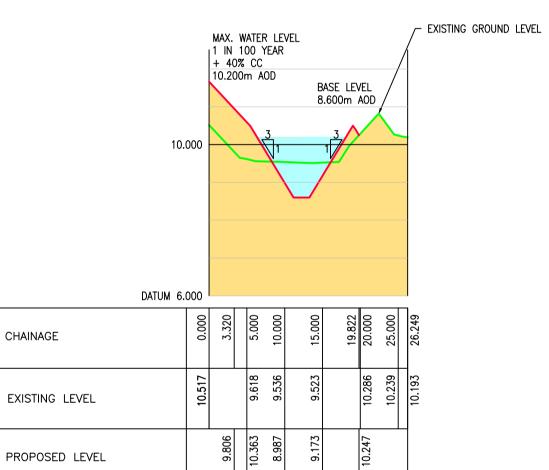




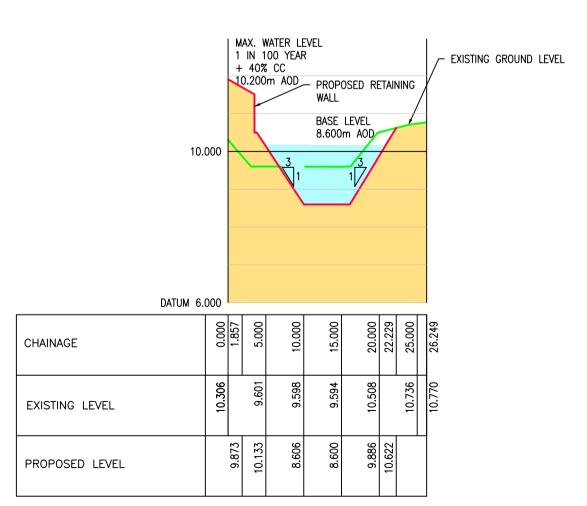


SWALE 11





SECTION E-E



SECTION F-F

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CONSTRUCTION

01	15.12.20	CONSTRUCTION ISSUE	TH/TH/AC
Γ2	07.10.20	PHASE 6B ADDED	KFM/TH/AC
Γ1	18.09.20	SWALES UPDATED.	CDU/TH/AC
P2	08.09.20	NEW SITE LAYOUT ADDED. SWALE 14 AMENDED. SWALE 14A REMOVED.	JJ/TH/AC
P1	11.08.20	FOR INFORMATION	JJ/TH/AC
Rev	Date	Revision Description	Drn/Chkd/Appd.

FAIRHURST

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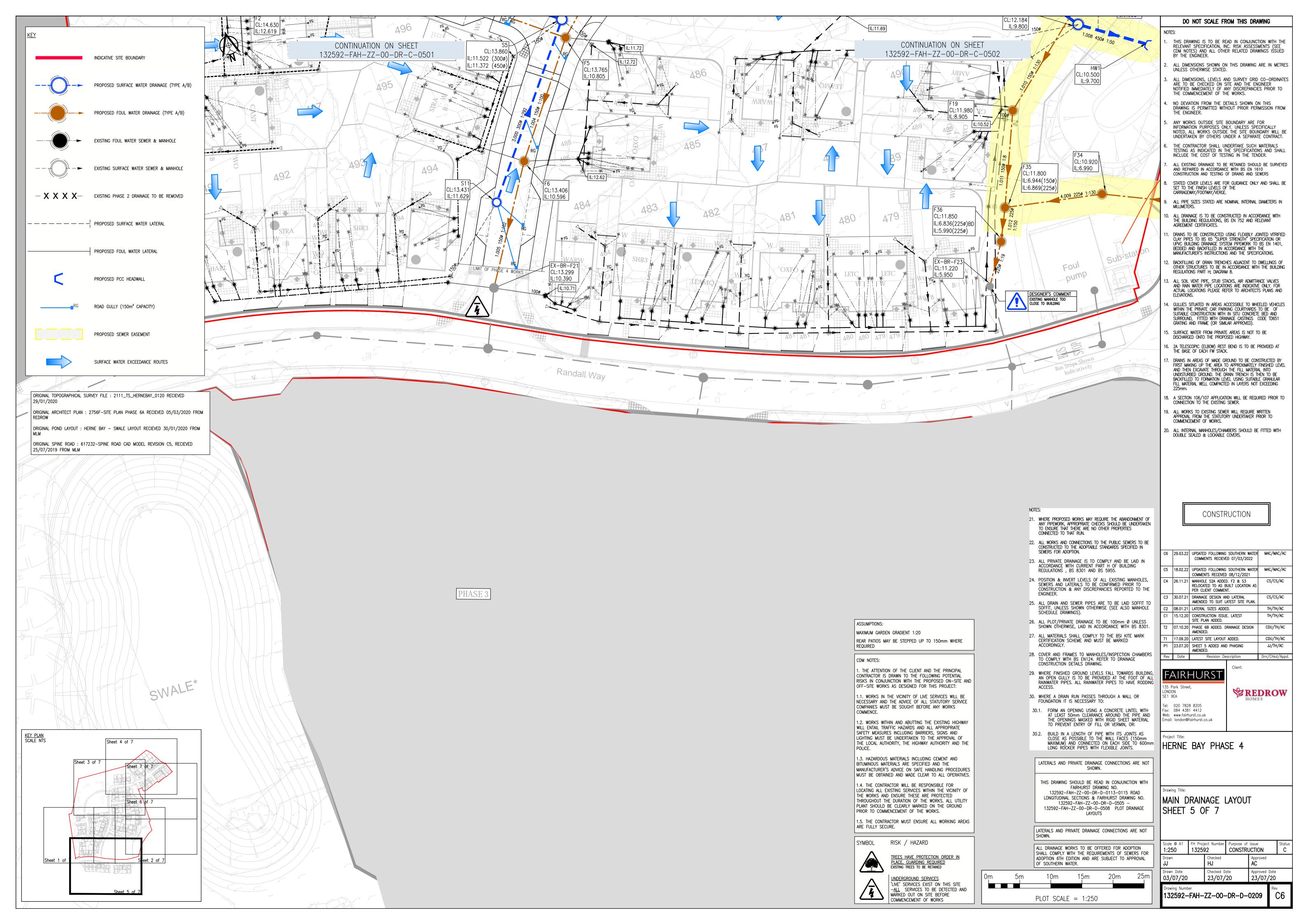
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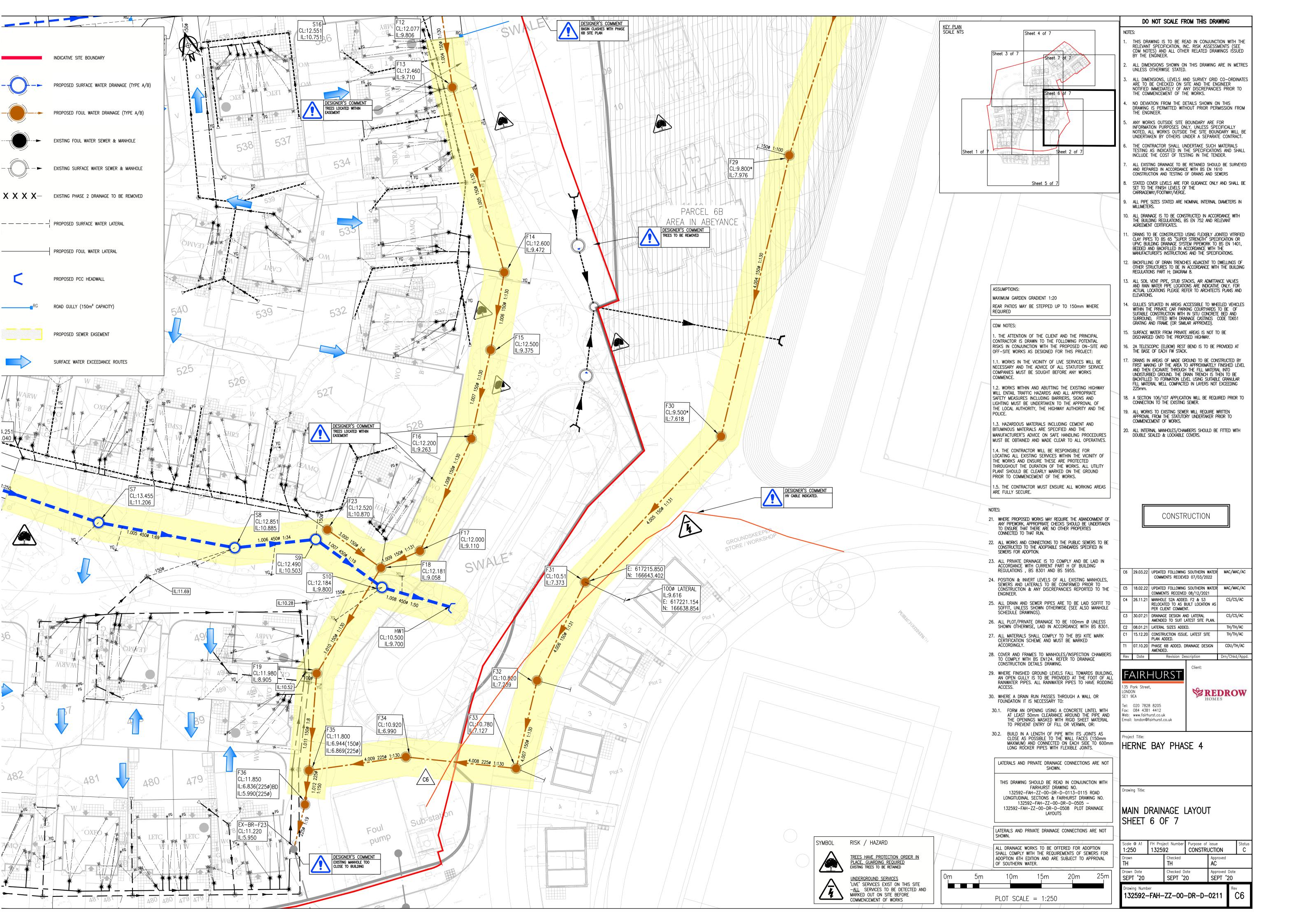
HERNE BAY PHASE 4

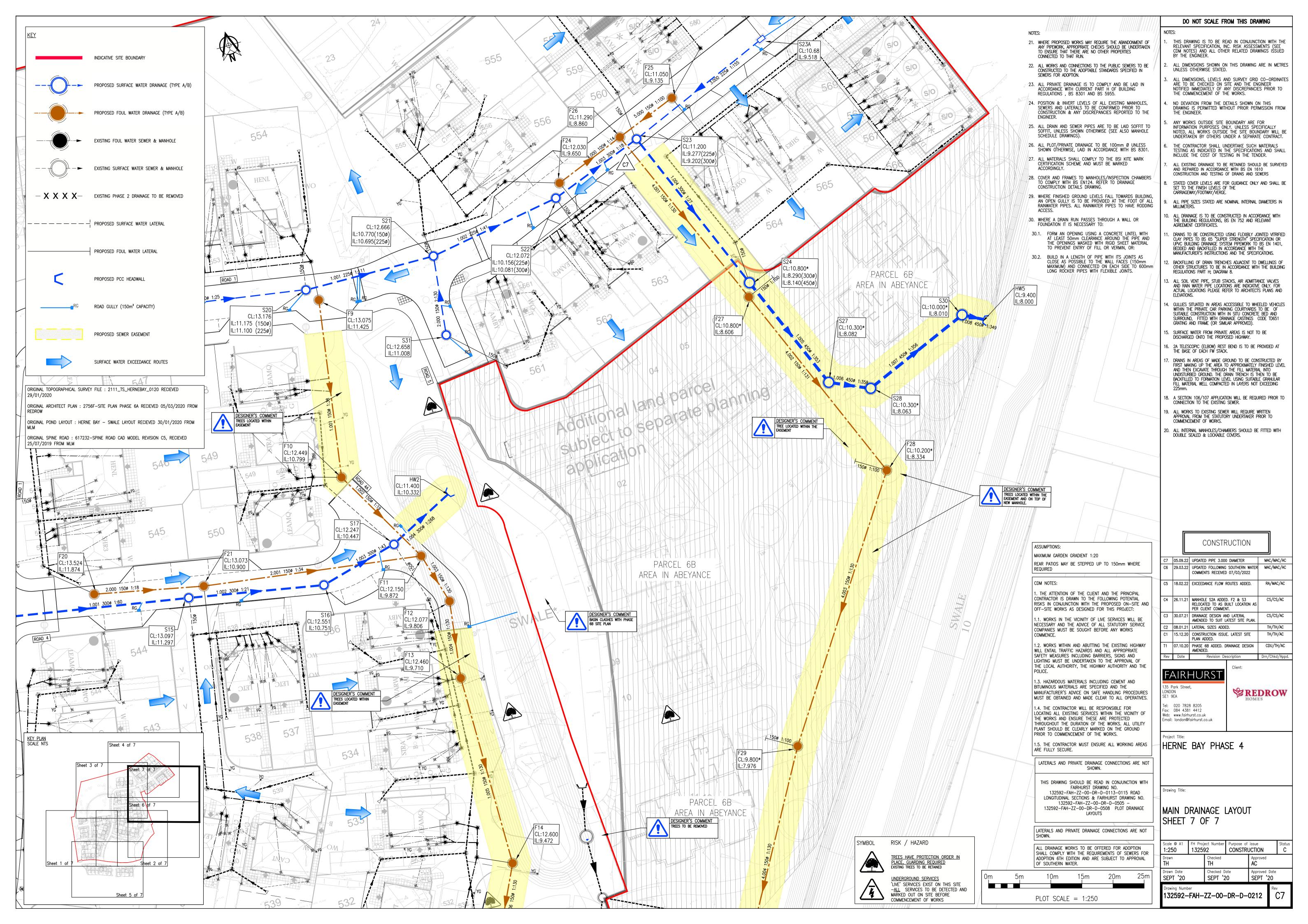
BASIN GENERAL ARRANGEMENT & SECTIONS SHEET 2 OF 2

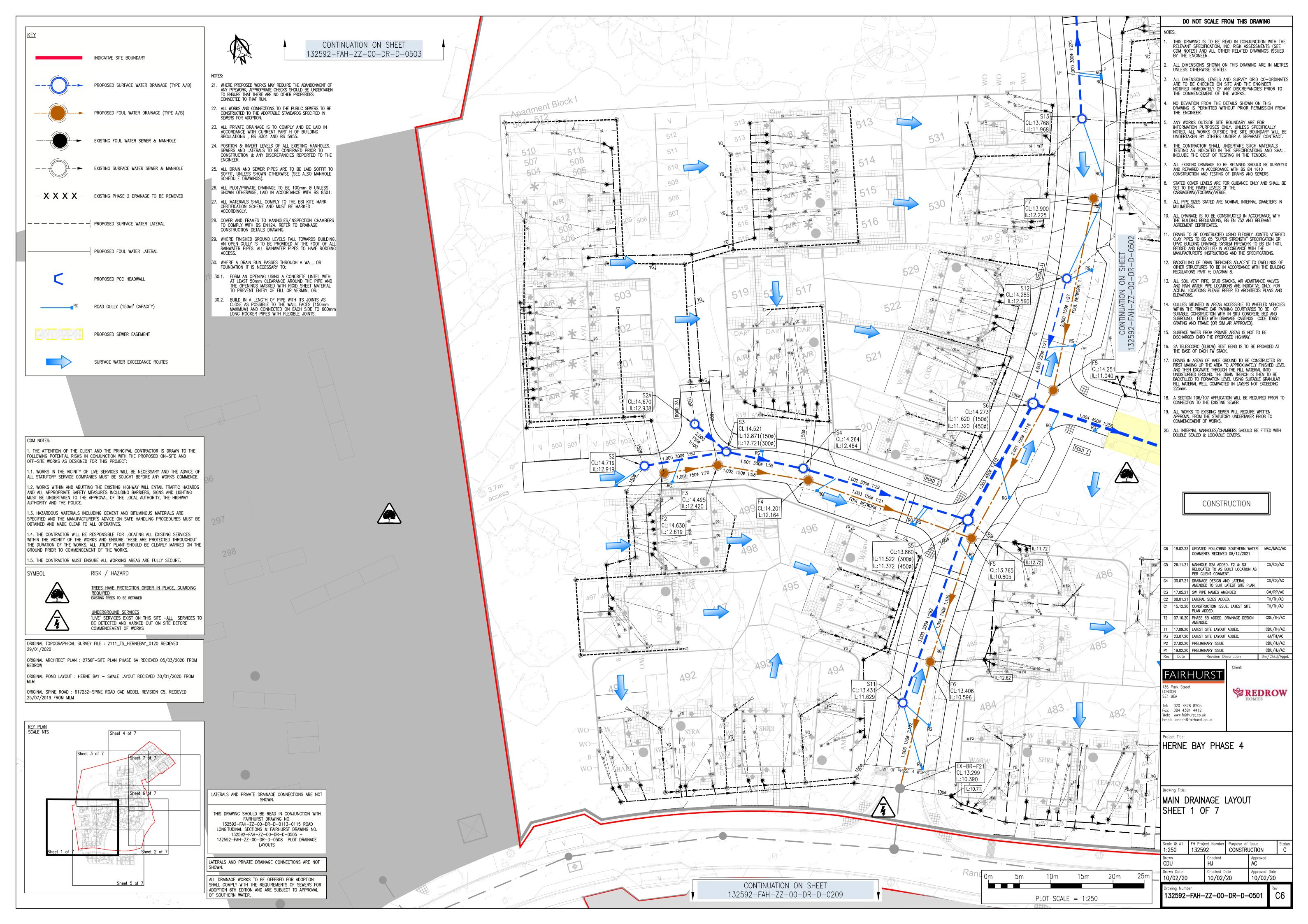
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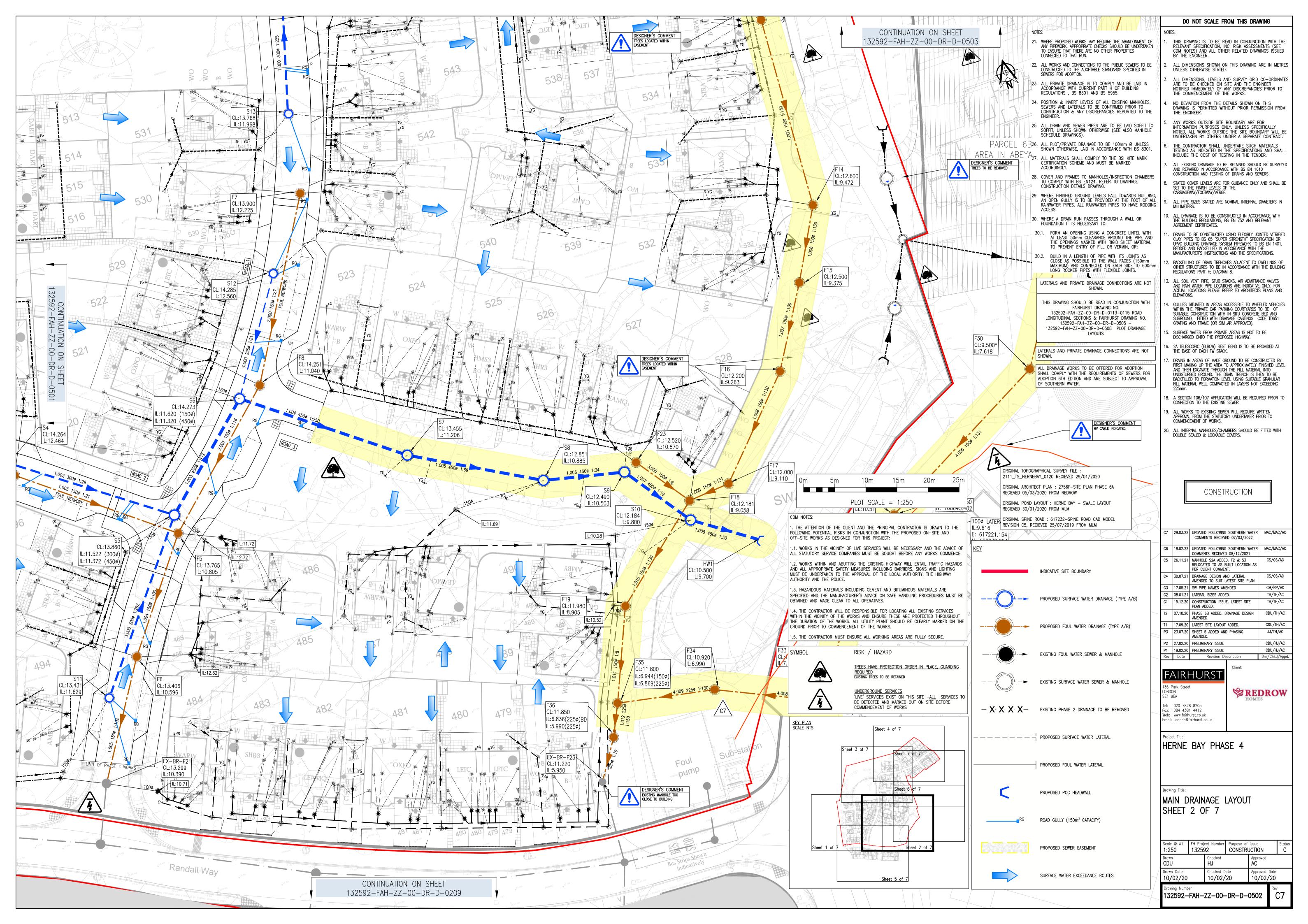
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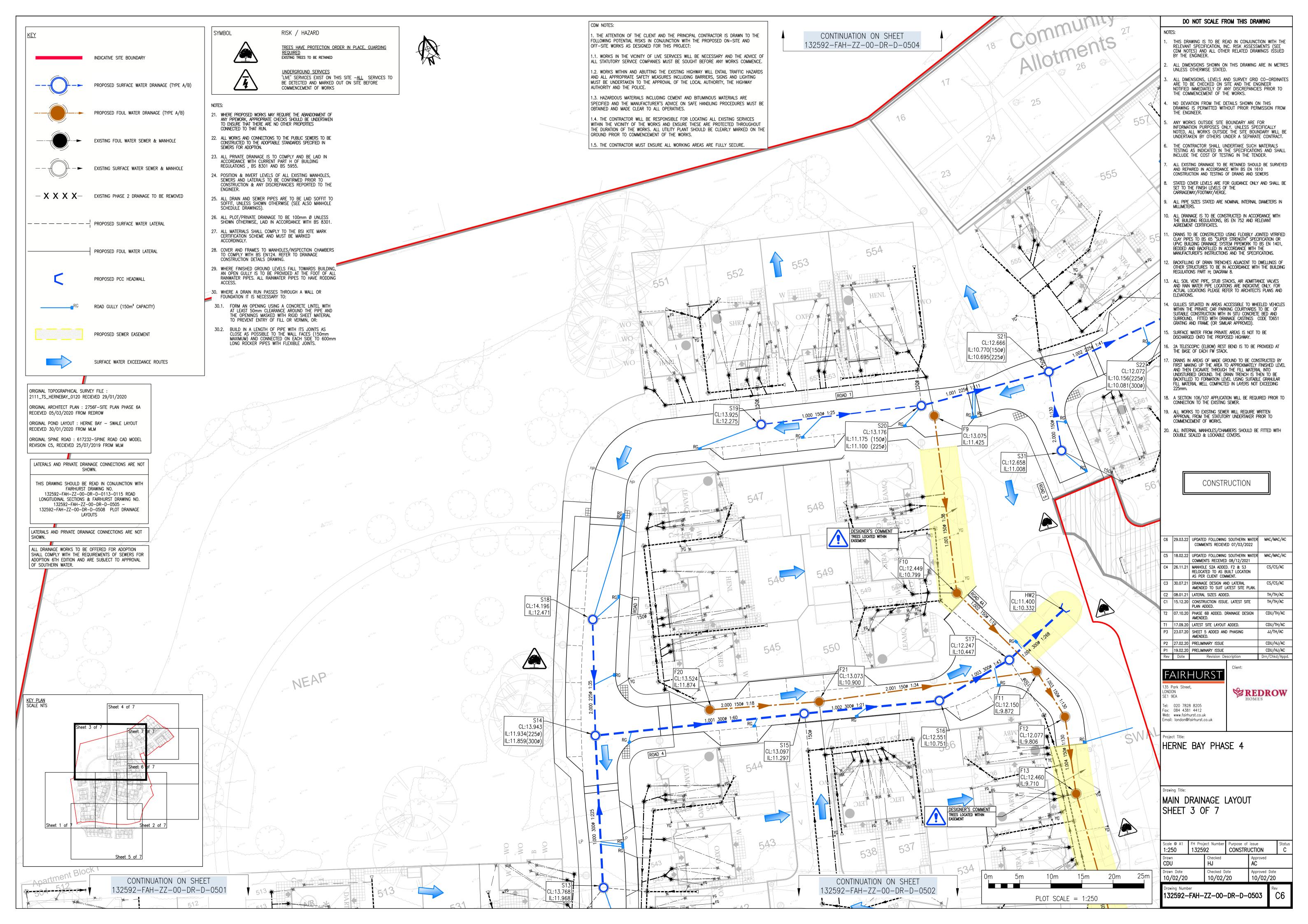


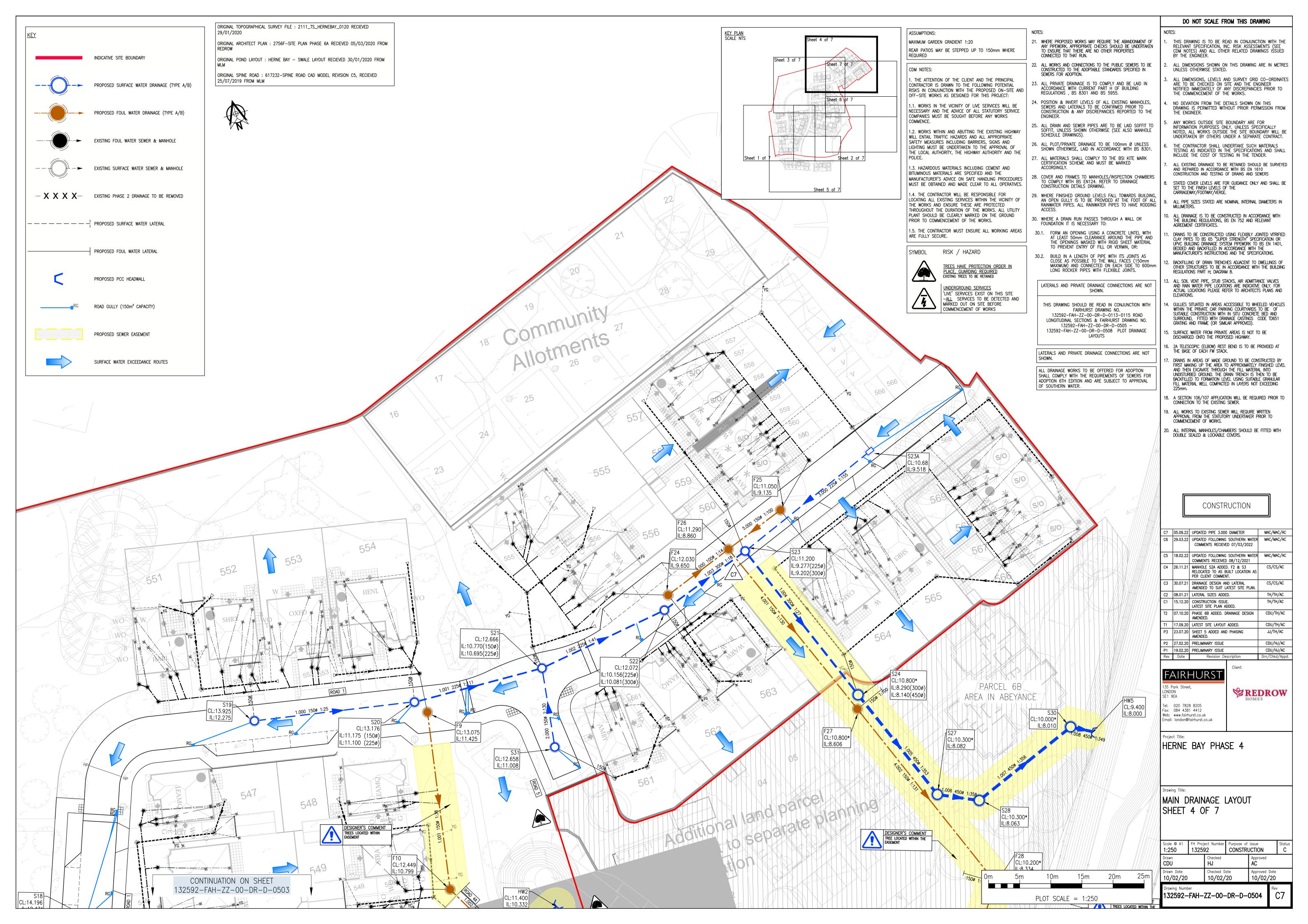






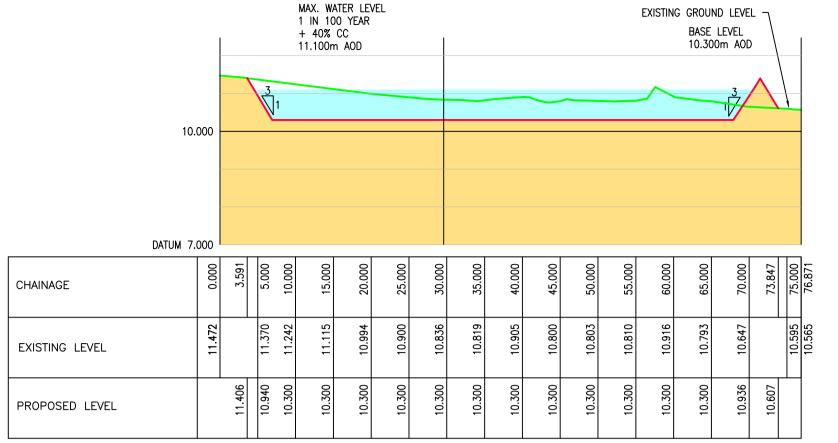




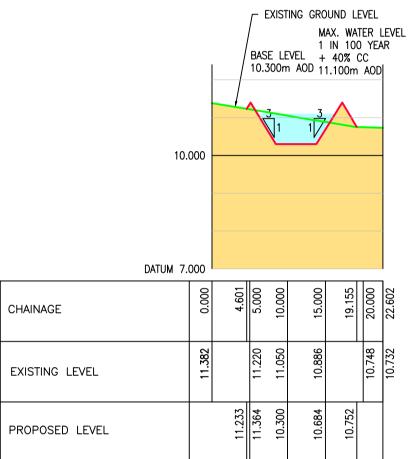




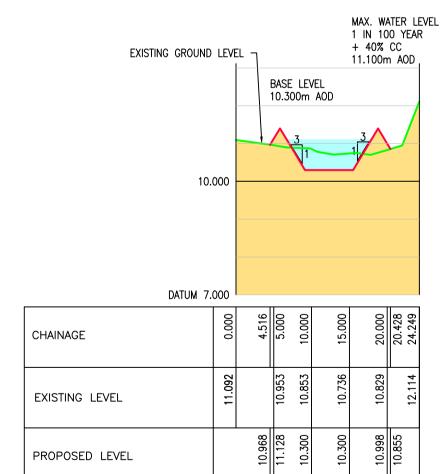
SWALE 14



SECTION A-A



SECTION B-B



SECTION C-C

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CONSTRUCTION

Rev	Date	Revision Description	Drn/Chkd/Appd.
P1	11.08.20	FOR INFORMATION	JJ/TH/AC
P2	08.09.20	NEW SITE LAYOUT ADDED. SWALE 14 AMENDED. SWALE 14A REMOVED.	JJ/TH/AC
T1	18.09.20	SWALES UPDATED.	CDU/TH/AC
T2	07.10.20	PHASE 6B ADDED	KFM/TH/AC
C1	15.12.20	CONSTRUCTION ISSUE	KFM/TH/AC

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REDROW

HERNE BAY PHASE 4

BASIN GENERAL ARRANGEMENT & SECTIONS SHEET 1 OF 2

4						
Scale @ A1 1:250	FH Proj.	ect Number 92	Purpose of CONSTRU		Status C	
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Drawn Date AUG 2020		Checked Date AUG 2020		Approved Date AUG 2020		

Drawing Number 132592—FAH—ZZ—00—DR—C—0525 C1



C. MicroDrainage Calculations

FOUL SEWERAGE DESIGN

Design Criteria for Foul - Main 2

Pipe Sizes FH-PIPE Manhole Sizes FH-MH

Industrial Flow (1/s/ha) 0.00 Add Flow / Climate Change (%) 0
Industrial Peak Flow Factor 0.00 Minimum Backdrop Height (m) 0.200
Flow Per Person (1/per/day) 222.00 Maximum Backdrop Height (m) 1.500
Persons per House 3.00 Min Design Depth for Optimisation (m) 1.200
Domestic (1/s/ha) 0.00 Min Vel for Auto Design only (m/s) 0.75
Domestic Peak Flow Factor 6.00 Min Slope for Optimisation (1:X) 500

Designed with Level Soffits

Network Design Table for Foul - Main 2

PN	Length (m)	Fall (m)	Slope (1:X)	Area (ha)	Houses	ase (1/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
F1.000	28.712	0.626	45.9	0.000	4	0.0	1.500	0	150	Pipe/Conduit	8
F1.001	17.988	0.927	19.4	0.000	3	0.0	1.500	0	150	Pipe/Conduit	ø.
F2.000	17.668	0.724	24.4	0.000	3	0.0	1.500	0	150	Pipe/Conduit	8
F2.001	35.289	1.278	27.6	0.000	4	0.0	1.500	0	150	Pipe/Conduit	er [®]
F1.002	8.583	0.066	130.0	0.000	0	0.0	1.500	0	150	Pipe/Conduit	ef [®]
F1.003	12.493	0.096	130.0	0.000	0	0.0	1.500	0	150	Pipe/Conduit	e e
F1.004	30.903	0.238	129.8	0.000	2	0.0	1.500	0	150	Pipe/Conduit	e e
F1.005	12.663	0.097	130.5	0.000	1	0.0	1.500	0	150	Pipe/Conduit	eri ^o
F1.006	14.586	0.112	130.2	0.000	1	0.0	1.500	0	150	Pipe/Conduit	œ [®]
F1.007	19.840	0.153	130.0	0.000	2	0.0	1.500	0	150	Pipe/Conduit	œ [®]
F1.008	6.807	0.052	130.9	0.000	0	0.0	1.500	0	150	Pipe/Conduit	₩
F3.000	10.581	1.812	5.8	0.000	3	0.0	1.500	0	150	Pipe/Conduit	8
F1.009	19.921	0.153	130.0	0.000	0	0.0	1.500	0	150	Pipe/Conduit	₽
F1.010	15.643	1.866	8.4	0.000	0	0.0	1.500	0	150	Pipe/Conduit	

Network Results Table

PN	US/IL (m)	Σ Area (ha)	Σ Base Flow (1/s)	Σ Hse	Add Flow (1/s)	P.Dep (mm)	P.Vel (m/s)	Vel (m/s)	Cap (1/s)	Flow (1/s)
F1.000	11.425	0.000	0.0	4	0.0	10	0.37	1.30	22.9	0.2
F1.001	10.799	0.000	0.0	7	0.0	11	0.60	1.99	35.3	0.3
F2.000	11.874	0.000	0.0	3	0.0	8	0.42	1.78	31.4	0.1
F2.001	11.150	0.000	0.0	7	0.0	11	0.53	1.67	29.5	0.3
F1.002	9.872	0.000	0.0	14	0.0	23	0.39	0.77	13.6	0.6
F1.003	9.806	0.000	0.0	14	0.0	23	0.39	0.77	13.6	0.6
F1.004	9.710	0.000	0.0	16	0.0	24	0.40	0.77	13.6	0.7
F1.005	9.472	0.000	0.0	17	0.0	25	0.41	0.77	13.5	0.8
F1.006	9.375	0.000	0.0	18	0.0	25	0.42	0.77	13.6	0.8
F1.007	9.263	0.000	0.0	20	0.0	27	0.43	0.77	13.6	0.9
F1.008	9.110	0.000	0.0	20	0.0	27	0.43	0.76	13.5	0.9
F3.000	10.870	0.000	0.0	3	0.0	5	0.67	3.64	64.3	0.1
F1.009	9.058	0.000	0.0	23	0.0	29	0.45	0.77	13.6	1.1
F1.010	8.905	0.000	0.0	23	0.0	15	1.17	3.04	53.7	1.1

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Innovyze	Network 2020.1	,

Network Design Table for Foul - Main 2

				_		_						
PN	Length	Fall	Slope	Area	Houses		ase	k	HYD	DIA	Section Type	Auto
	(m)	(m)	(1:X)	(ha)		Flow	(1/s)	(mm)	SECT	(mm)		Design
F4.000	12.185	0.790	15.4	0.000	5		0.0	1.500	0	150	Pipe/Conduit	ð
F5.000	10.457	0.275	38.0	0.000	13		0.0	1.500	0	150	Pipe/Conduit	8
F4.001	33.007	0.254	129.9	0.000	0		0.0	1.500	0	150	Pipe/Conduit	F
F4.002	35.607	0.272	130.9	0.000	2		0.0	1.500	0	150	Pipe/Conduit	ď
F4.003	46.603	0.358	130.2	0.000	6		0.0	1.500	0	150	Pipe/Conduit	ď
F4.004	46.483	0.358	129.8	0.000	5		0.0	1.500	0	150	Pipe/Conduit	ď
F4.005	33.857	0.260	130.2	0.000	0		0.0	1.500	0	150	Pipe/Conduit	ď
F4.006	15.493	0.119	130.2	0.000	1		0.0	1.500	0	150	Pipe/Conduit	ď
F4.007	14.513	0.112	129.6	0.000	1		0.0	1.500	0	150	Pipe/Conduit	ď
F4.008	17.808	0.137	130.0	0.000	2		0.0	1.500	0	150	Pipe/Conduit	ď
F4.009	15.743	0.121	130.1	0.000	0		0.0	1.500	0	150	Pipe/Conduit	ď
F1.011	5.514	1.049	5.3	0.000	0		0.0	1.500	0	150	Pipe/Conduit	ø.
F1.012	7.400	0.050	148.0	0.000	0		0.0	1.500	0	150	Pipe/Conduit	⊕

Network Results Table

PN	US/IL (m)	Σ Area (ha)	Σ Base Flow (1/s)	Σ Hse	Add Flow (1/s)	P.Dep (mm)	P.Vel (m/s)	Vel (m/s)	Cap (1/s)	Flow (1/s)	
F4.000	9.650	0.000	0.0	5	0.0	9	0.58	2.24	39.6	0.2	
F5.000	9.135	0.000	0.0	13	0.0	16	0.58	1.42	25.2	0.6	
F4.001	8.860	0.000	0.0	18	0.0	25	0.42	0.77	13.6	0.8	
F4.002	8.606	0.000	0.0	20	0.0	27	0.43	0.76	13.5	0.9	
F4.003	8.334	0.000	0.0	26	0.0	30	0.47	0.77	13.6	1.2	
F4.004	7.976	0.000	0.0	31	0.0	33	0.50	0.77	13.6	1.4	
F4.005	7.618	0.000	0.0	31	0.0	33	0.49	0.77	13.6	1.4	
F4.006	7.358	0.000	0.0	32	0.0	34	0.50	0.77	13.6	1.5	
F4.007	7.239	0.000	0.0	33	0.0	34	0.50	0.77	13.6	1.5	
F4.008	7.127	0.000	0.0	35	0.0	35	0.51	0.77	13.6	1.6	
F4.009	6.990	0.000	0.0	35	0.0	35	0.51	0.77	13.6	1.6	
F1.011	6.869	0.000	0.0	58	0.0	21	1.83	3.84	67.8	2.7	
F1.012	5.990	0.000	0.0	58	0.0	47	0.57	0.72	12.7	2.7	

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$\underline{\text{Manhole Schedules for Foul - Main 2}}$

MH Name	MH CL (m)	MH Depth (m)	Coni	MH nection	MH Diam.,L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Backdrop (mm)
F9	13.075	1.650	Open	Manhole	1200	F1.000	11.425	150				
F10	12.408	1.609	Open	Manhole	1200	F1.001	10.799	150	F1.000	10.799	150	
F20	13.524	1.650	Open	Manhole	1200	F2.000	11.874	150				
F21	13.073	1.923	Open	Manhole	1200	F2.001	11.150	150	F2.000	11.150	150	
F11	12.150	2.278	Open	Manhole	1200	F1.002	9.872	150	F1.001	9.872	150	
									F2.001	9.872	150	
F12	12.077	2.271	Open	Manhole	1200	F1.003	9.806	150	F1.002	9.806	150	
F13	12.460	2.750	Open	Manhole	1200	F1.004	9.710	150	F1.003	9.710	150	
F14	12.600	3.128	_	Manhole	1200	F1.005	9.472	150	F1.004	9.472	150	
F15			Open	Manhole	1200	F1.006	9.375	150	F1.005	9.375	150	
F16			Open	Manhole	1200	F1.007	9.263	150	F1.006	9.263	150	
F17	12.000	2.890	Open	Manhole	1200	F1.008	9.110	150	F1.007	9.110	150	
F23	12.520	1.650	Open	Manhole	1200	F3.000	10.870	150				
F18	12.181	3.123	Open	Manhole	1200	F1.009	9.058	150	F1.008	9.058	150	
									F3.000	9.058	150	
F19	11.980	3.075	Open	Manhole	1200	F1.010	8.905	150	F1.009	8.905	150	
F24	12.030	2.380	Open	Manhole	1200	F4.000	9.650	150				
F25	11.050		-	Manhole	1200	F5.000	9.135	150				
F26	11.290	2.430	Open	Manhole	1200	F4.001	8.860	150	F4.000	8.860	150	
									F5.000	8.860	150	
F27	10.800	2.194	Open	Manhole	1200	F4.002	8.606	150	F4.001	8.606	150	
F28	10.200	1.866	Open	Manhole	1200	F4.003	8.334	150	F4.002	8.334	150	
F29	9.800	1.824	Open	Manhole	1200	F4.004	7.976	150	F4.003	7.976	150	
F30	9.500	1.882	Open	Manhole	1200	F4.005	7.618	150	F4.004	7.618	150	
F31	10.670	3.312	Open	Manhole	1200	F4.006	7.358	150	F4.005	7.358	150	
F32	10.820	3.581	Open	Manhole	1200	F4.007	7.239	150	F4.006	7.239	150	
F33	10.780	3.653	Open	Manhole	1200	F4.008	7.127	150	F4.007	7.127	150	
F34	10.920	3.930	Open	Manhole	1200	F4.009	6.990	150	F4.008	6.990	150	
F35	11.800	4.931	Open	Manhole	1200	F1.011	6.869	150	F1.010	7.039	150	170
									F4.009	6.869	150	
F36	11.850	6.030	Open	Manhole	1200	F1.012	5.990	150	F1.011	5.820	150	
FBRF23	11.220	5.280	Open	Manhole	0		OUTFALL		F1.012	5.940	150	

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
F9	617212.072	166790.432	617212.072	166790.432	Required	
F10	617207.728	166762.051	617207.728	166762.051	Required	
F20	617164.333	166754.953	617164.333	166754.953	Required	•
F21	617181.629	166751.348	617181.629	166751.348	Required	

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Manhole Schedules for Foul - Main 2

МН	Manhole	Manhole		Intersection		Layout
Name	Easting (m)	Northing (m)	Easting (m)	Northing (m)	Access	(North)
F11	617216.567	166746.384	617216.567	166746.384	Required	
F12	617218.936	166738.135	617218.936	166738.135	Required	1
F13	617217.259	166725.755	617217.259	166725.755	Required	1
F14	617217.066	166694.853	617217.066	166694.853	Required	
F15	617211.527	166683.465	617211.527	166683.465	Required	1
F16	617204.619	166670.619	617204.619	166670.619	Required	7
F17	617191.712	166655.552	617191.712	166655.552	Required	-6
F23	617178.330	166662.934	617178.330	166662.934	Required	0
F18	617184.960	166654.688	617184.960	166654.688	Required	-
F19	617170.311	166641.188	617170.311	166641.188	Required	0
F24	617254.460	166797.867	617254.460	166797.867	Required	-
F25	617275.575	166806.041	617275.575	166806.041	Required	
F26	617265.815	166802.287	617265.815	166802.287	Required	-0-
F27	617278.655	166771.880	617278.655	166771.880	Required	
F28	617292.294	166738.988	617292.294	166738.988	Required	1
F29	617266.300	166700.309	617266.300	166700.309	Required	
F30	617243.316	166659.905	617243.316	166659.905	Required	
F31	617215.103	166641.189	617215.103	166641.189	Required	0
F32	617204.054	166630.328	617204.054	166630.328	Required	8
						/

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Innovyze	Network 2020.1	1

Manhole Schedules for Foul - Main 2

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
F33	617196.924	166617.687	617196.924	166617.687	Required	4
F34	617180.404	166624.338	617180.404	166624.338	Required	-0.,
F35	617164.816	166626.542	617164.816	166626.542	Required	ø
F36	617162.694	166621.452	617162.694	166621.452	Required	1
FBRF23	617158.982	166615.050			No Entry	1

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PIPELINE SCHEDULES for Foul - Main 2

Upstream Manhole

PN	-	Diam (mm)	MH Name	C.Level (m)	I.Level	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
F1.000	0	150	F9	13.075	11.425	1 500	Open Manhole	1200
F1.000	0	150	F10	12.408	10.799		Open Manhole	1200
F2.000	0	150	F20	13.524	11.874	1.500	Open Manhole	1200
F2.001	0	150	F21	13.073	11.150	1.773	Open Manhole	1200
F1.002	0	150	F11	12.150	9.872		Open Manhole	1200
F1.003	0	150	F12	12.077	9.806	2.121	Open Manhole	1200
F1.004	0	150	F13	12.460	9.710	2.600	Open Manhole	1200
F1.005	0	150	F14	12.600	9.472	2.978	Open Manhole	1200
F1.006	0	150	F15	12.500	9.375	2.975	Open Manhole	1200
F1.007	0	150	F16	12.200	9.263	2.787	Open Manhole	1200
F1.008	0	150	F17	12.000	9.110	2.740	Open Manhole	1200
F3.000	0	150	F23	12.520	10.870	1.500	Open Manhole	1200
F1.009	0	150	F18	12.181	9.058	2.973	Open Manhole	1200
F1.010	0	150	F19	11.980	8.905		Open Manhole	1200
F4.000	0	150	F24	12.030	9.650	2.230	Open Manhole	1200
F5.000	0	150	F25	11.050	9.135	1.765	Open Manhole	1200
F4.001	0	150	F26	11.290	8.860	2.280	Open Manhole	1200
F4.002	0	150	F27	10.800	8.606	2.044	Open Manhole	1200
F4.003	0	150	F28	10.200	8.334	1.716	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)		C.Level (m)	I.Level (m)	D.Depth (m)		MH nection	МН	DIAM., (mm)	L*W
F1.000	28.712	45.9	F10	12.408	10.799	1.459	Open	Manhole		1	L200
	17.988						-	Manhole			L200
F2.000	17.668	24.4	F21	13.073	11.150	1.773	Open	Manhole		1	L200
F2.001	35.289	27.6	F11	12.150	9.872	2.128	Open	Manhole		1	L200
F1.002	8.583	130.0	F12	12.077	9.806	2.121	Open	Manhole		1	L200
F1.003	12.493	130.0	F13	12.460	9.710	2.600	Open	Manhole		1	L200
F1.004	30.903	129.8	F14	12.600	9.472	2.978	Open	Manhole		1	L200
F1.005	12.663	130.5	F15	12.500	9.375			Manhole		1	L200
F1.006	14.586	130.2	F16	12.200	9.263	2.787	Open	Manhole		1	L200
F1.007	19.840	130.0	F17	12.000	9.110	2.740	Open	Manhole		1	L200
F1.008	6.807	130.9	F18	12.181	9.058	2.973	Open	Manhole		1	L200
F3.000	10.581	5.8	F18	12.181	9.058	2.973	Open	Manhole		1	L200
F1.009	19.921	130.0	F19	11.980	8.905	2.925	Open	Manhole		1	L200
F1.010	15.643	8.4	F35	11.800	7.039	4.611	Open	Manhole		1	L200
F4.000	12.185	15.4	F26	11.290	8.860	2.280	Open	Manhole		1	L200
F5.000	10.457	38.0	F26	11.290	8.860	2.280	Open	Manhole		1	L200
F4.001	33.007	129.9	F27	10.800	8.606	2.044	Open	Manhole		1	L200
F4.002	35.607	130.9	F28	10.200	8.334		-	Manhole		1	L200
F4.003	46.603	130.2	F29	9.800	7.976	1.674	Open	Manhole		1	L200

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PIPELINE SCHEDULES for Foul - Main 2

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
F4.004	0	150	F29	9.800	7.976	1.674	Open Manhole	1200
F4.005	0	150	F30	9.500	7.618	1.732	Open Manhole	1200
F4.006	0	150	F31	10.670	7.358	3.162	Open Manhole	1200
F4.007	0	150	F32	10.820	7.239	3.431	Open Manhole	1200
F4.008	0	150	F33	10.780	7.127	3.503	Open Manhole	1200
F4.009	0	150	F34	10.920	6.990	3.780	Open Manhole	1200
F1.011	0	150	F35	11.800	6.869	4.781	Open Manhole	1200
F1.012	0	150	F36	11.850	5.990	5.710	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
	(III)	(I:A)	Name	(III)	(111)	(111)	Connection	(mun)
F4.004	46.483	129.8	F30	9.500	7.618	1.732	Open Manhole	1200
F4.005	33.857	130.2	F31	10.670	7.358	3.162	Open Manhole	1200
F4.006	15.493	130.2	F32	10.820	7.239	3.431	Open Manhole	1200
F4.007	14.513	129.6	F33	10.780	7.127	3.503	Open Manhole	1200
F4.008	17.808	130.0	F34	10.920	6.990	3.780	Open Manhole	1200
F4.009	15.743	130.1	F35	11.800	6.869	4.781	Open Manhole	1200
F1.011	5.514	5.3	F36	11.850	5.820	5.880	Open Manhole	1200
F1.012	7.400	148.0	FBRF23	11,220	5.940	5.130	Open Manhole	0

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STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm 1

Pipe Sizes FH-PIPE Manhole Sizes FH-MH

FSR Rainfall Model - England and Wales

Return Period (years) 30 PIMP (%) 100
M5-60 (mm) 26.250 Add Flow / Climate Change (%) 0
Ratio R 0.400 Minimum Backdrop Height (m) 0.200
Maximum Rainfall (mm/hr) 50 Maximum Backdrop Height (m) 1.500
Maximum Time of Concentration (mins) 30 Min Design Depth for Optimisation (m) 1.200
Foul Sewage (1/s/ha) 0.000 Min Vel for Auto Design only (m/s) 1.00
Volumetric Runoff Coeff. 0.750 Min Slope for Optimisation (1:X) 500

Designed with Level Soffits

Time Area Diagram for Storm 1

Time Area (mins) (ha) (mins) (ha) (ha) 0-4 0.556 4-8 0.235

Total Area Contributing (ha) = 0.791

Total Pipe Volume $(m^3) = 31.271$

Network Design Table for Storm 1

Auto Design	Section Type	DIA (mm)	HYD SECT	k (mm)	ase (1/s)	T.E. (mins)	I.Area (ha)	Slope (1:X)	Fall (m)	Length (m)	PN
8	Pipe/Conduit	300	0	0.600	0.0	5.00	0.077	67.4	0.198	13.339	S1.000
ð	Pipe/Conduit	150	0	0.600	0.0	5.00	0.034	100.4	0.067	6.724	S2.000
0	Pipe/Conduit Pipe/Conduit		0	0.600		0.00	0.095 0.027	44.1 29.4		12.668 27.729	
•	Pipe/Conduit	300	0	0.600	0.0	5.00	0.080	291.6	0.107	31.202	s3.000
&	Pipe/Conduit	450	0	0.600	0.0	0.00	0.016	412.5	0.052	21.448	S1.003
6	Pipe/Conduit	225	0	0.600	0.0	5.00	0.091	20.5	1.015	20.822	S4.000

Network Results Table

PN	Rain (mm/hr)	T.C.	US/IL (m)		Σ Base Flow (1/s)		Add Flow (1/s)	Vel (m/s)	Cap (1/s)	Flow (1/s)
S1.000	50.00	5.12	12.919	0.077	0.0	0.0	0.0	1.92	135.6	10.4
S2.000	50.00	5.11	12.938	0.034	0.0	0.0	0.0	1.00	17.7	4.6
S1.001 S1.002	50.00 50.00		12.721 12.464	0.206 0.233	0.0	0.0	0.0		167.7 205.6	27.9 31.6
s3.000	50.00	5.57	11.629	0.080	0.0	0.0	0.0	0.92	64.7	10.8
S1.003	50.00	5.93	11.372	0.329	0.0	0.0	0.0	0.99	158.2	44.6
S4.000	50.00	5.12	12.560	0.091	0.0	0.0	0.0	2.90	115.4	12.3

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Network Design Table for Storm 1

\$1.004 28.441 0.114 249.5 0.083 0.00 0.0 0.600 o 450 Pipe/Conduit \$1.005 22.265 0.321 69.4 0.005 0.00 0.0 0.600 o 450 Pipe/Conduit \$1.006 13.083 0.382 34.2 0.015 0.00 0.0 0.600 o 450 Pipe/Conduit \$1.007 9.704 0.703 13.8 0.039 0.00 0.0 0.600 o 450 Pipe/Conduit \$1.008 9.891 0.100 98.9 0.020 0.00 0.0 0.600 o 450 Pipe/Conduit \$1.009 26.139 1.100 23.8 0.208 0.00 0.0 0.600 o 450 Pipe/Conduit \$1.009 26.139 1.100 23.8 0.208 0.00 0.0 0.600 o 450 Pipe/Conduit \$1.009 26.139 1.100 23.8 0.208 0.00 0.0 0.600 o 450 Pipe/Conduit \$1.009 26.139 1.100 23.8 0.208 0.00 0.0 0.600 o 450 Pipe/Conduit \$1.009 26.139 1.100 23.8 0.208 0.00 0.00 0.000 0.000 o 450 Pipe/Conduit \$1.009 26.139 1.100 23.8 0.208 0.00 0.00 0.000 0.000 o 450 Pipe/Conduit \$1.009 26.139 1.100 23.8 0.208 0.00 0.000 0.000 0.000 o 450 Pipe/Conduit \$1.009 26.139 1.100 23.8 0.208 0.00 0.000 0.000 0.000 o 450 Pipe/Conduit \$1.009 26.139 1.100 23.8 0.208 0.00 0.000 0.000 0.000 o 450 Pipe/Conduit \$1.009 26.139 1.100 23.8 0.208 0.000 0.000 0.000 0.000 o 450 Pipe/Conduit \$1.009 26.139 1.100 23.8 0.208 0.000 0.000 0.000 0.000 o 450 Pipe/Conduit \$1.009 26.139 1.100 23.8 0.208 0.000 0.000 0.000 0.000 0.000 o 450 Pipe/Conduit \$1.009 26.139 1.100 23.8 0.208 0.000 0.000 0.000 0.000 o 450 Pipe/Conduit \$1.009 26.139 1.100 0.0	PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)		Base Flow (1/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
\$1.010 6.362 0.030 212.1 0.000 0.00 0.0 0.600 o 450 Pipe/Conduit \$\mathbb{g}\$\$ \$1.011 15.582 0.520 30.0 0.000 0.00 0.00 0.600 o 450 Pipe/Conduit	\$1.005 \$1.006 \$1.007 \$1.008 \$1.009	22.265 13.083 9.704 9.891 26.139	0.321 0.382 0.703 0.100 1.100	69.4 34.2 13.8 98.9 23.8	0.005 0.015 0.039 0.020 0.208	0.00 0.00 0.00 0.00	0.0 0.0 0.0 0.0	0.600 0.600 0.600 0.600 0.600	0 0 0	450 450 450 450 450	Pipe/Conduit Pipe/Conduit Pipe/Conduit Pipe/Conduit Pipe/Conduit	######################################

Network Results Table

PN	Rain	T.C.	US/IL	Σ I.Area	Σ Base	Foul	Add Flow	Vel	Cap	Flow
	(mm/hr)	(mins)	(m)	(ha)	Flow $(1/s)$	(1/s)	(1/s)	(m/s)	(1/s)	(1/s)
S1.004	50.00	6.30	11.320	0.504	0.0	0.0	0.0	1.28	204.0	68.2
S1.005	50.00	6.45	11.206	0.509	0.0	0.0	0.0	2.44	388.7	68.9
S1.006	50.00	6.51	10.885	0.524	0.0	0.0	0.0	3.48	554.0	70.9
S1.007	50.00	6.54	10.503	0.563	0.0	0.0	0.0	5.49	873.8	76.2
S1.008	50.00	6.62	9.800	0.583	0.0	0.0	0.0	2.04	325.1	78.9
S1.009	50.00	6.73	9.700	0.791	0.0	0.0	0.0	4.18	665.5	107.1
S1.010	50.00	6.80	8.600	0.791	0.0	0.0	0.0	1.39	221.4	107.1
S1.011	50.00	6.87	8.570	0.791	0.0	0.0	0.0	3.72	592.4	107.1
S1.005 S1.006 S1.007 S1.008 S1.009 S1.010	50.00 50.00 50.00 50.00 50.00 50.00	6.45 6.51 6.54 6.62 6.73 6.80	11.206 10.885 10.503 9.800 9.700 8.600	0.524 0.563 0.583 0.791 0.791	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	2.44 3.48 5.49 2.04 4.18 1.39	554.0 873.8 325.1 665.5 221.4	68.9 70.9 76.2 78.9 107.1

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Manhole Schedules for Storm 1

MH Name	MH CL (m)	MH Depth (m)	Coni	MH nection	MH Diam.,L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Backdrop (mm)
S2	14.719	1.800	Open	Manhole	1200	S1.000	12.919	300				
S2A	14.670	1.732	Open	Manhole	1200	s2.000	12.938	150				
s3	14.521	1.800	Open	Manhole	1200	S1.001	12.721	300	S1.000	12.721	300	
									S2.000	12.871	150	
S4	14.264	1.830	Open	Manhole	1200	S1.002	12.464	300	S1.001	12.434	300	
S11	13.431	1.802	Open	Manhole	1200	s3.000	11.629	300				
S5	13.860	2.488	Open	Manhole	1500	s1.003	11.372	450	S1.002	11.522	300	
									s3.000	11.522	300	
S12	14.285	1.725	Open	Manhole	1200	S4.000	12.560	225				
S6	14.273	2.953	Open	Manhole	1500	s1.004	11.320	450	s1.003	11.320	450	
									S4.000	11.545	225	
s7	13.455	2.249	Open	Manhole	1500	s1.005	11.206	450	S1.004	11.206	450	
S8	12.851	1.966	Open	Manhole	1500	S1.006	10.885	450	S1.005	10.885	450	
S9	12.490	1.987	Open	Manhole	1500	S1.007	10.503	450	S1.006	10.503	450	
S10	12.184	2.384	Open	Manhole	1500	s1.008	9.800	450	S1.007	9.800	450	
SHW1	10.500	0.800	Open	Manhole	1500	s1.009	9.700	450	S1.008	9.700	450	
SSW11	10.500	1.900	Open	Manhole	1500	S1.010	8.600	450	S1.009	8.600	450	
SHW2	10.500	1.930	Open	Manhole	1500	s1.011	8.570	450	S1.010	8.570	450	
S25	11.640	3.590	Open	Manhole	0		OUTFALL		S1.011	8.050	450	

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S2	617057.093	166697.925	617057.093	166697.925	Required	0
S2A	617066.722	166702.171	617066.722	166702.171	Required	9
S3	617070.357	166696.515	617070.357	166696.515	Required	-6
S4	617081.511	166690.508	617081.511	166690.508	Required	-0
S11	617086.507	166649.868	617086.507	166649.868	Required	6
S5	617104.662	166675.244	617104.662	166675.244	Required	1
S12	617130.650	166708.169	617130.650	166708.169	Required	,
S6	617119.904	166690.334	617119.904	166690.334	Required	1
s7	617143.308	166674.174	617143.308	166674.174	Required	-

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Manhole Schedules for Storm 1

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S8	617163.234	166664.239	617163.234	166664.239	Required	-0-
S9	617176.114	166661.942	617176.114	166661.942	Required	
S10	617182.103	166654.308	617182.103	166654.308	Required	10
SHW1	617188.202	166646.520	617188.202	166646.520	Required	1
SSW11	617211.368	166658.628	617211.368	166658.628	Required	.6
SHW2	617216.149	166662.824	617216.149	166662.824	Required	,6
S25	617225.209	166675.502			No Entry	

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PIPELINE SCHEDULES for Storm 1

Upstream Manhole

PN	Hyd Sect	Diam	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
	5000	(11111)	rome	(,	(1117)	(/	0011110002011	(11111)
S1.000	0	300	S2	14.719	12.919	1.500	Open Manhole	1200
s2.000	0	150	S2A	14.670	12.938	1.582	Open Manhole	1200
S1.001 S1.002	0	300 300	S3 S4		12.721 12.464		Open Manhole Open Manhole	
s3.000	0	300	S11	13.431	11.629	1.502	Open Manhole	1200
S1.003	0	450	S5	13.860	11.372	2.038	Open Manhole	1500
S4.000	0	225	S12	14.285	12.560	1.500	Open Manhole	1200
S1.004	0	450	S6	14.273	11.320	2.503	Open Manhole	1500
S1.005	0	450	s7	13.455	11.206	1.799	Open Manhole	1500
S1.006	0	450	S8	12.851	10.885	1.516	Open Manhole	1500
S1.007	0	450	s9	12.490	10.503		Open Manhole	
S1.008	0	450	S10	12.184	9.800		Open Manhole	
S1.009	0	450	SHW1	10.500	9.700		Open Manhole	
S1.010	0	450	SSW11	10.500	8.600		Open Manhole	
S1.011	0	450	SHW2	10.500	8.570		Open Manhole	

Downstream Manhole

PN	Length (m)	Slope (1:X)		C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.000	13.339	67.4	s3	14.521	12.721	1.500	Open Manhole	1200
S2.000	6.724	100.4	S3	14.521	12.871	1.500	Open Manhole	1200
	12.668 27.729		S4 S5	14.264 13.860			Open Manhole Open Manhole	
s3.000	31.202	291.6	S5	13.860	11.522	2.038	Open Manhole	1500
S1.003	21.448	412.5	S6	14.273	11.320	2.503	Open Manhole	1500
S4.000	20.822	20.5	S6	14.273	11.545	2.503	Open Manhole	1500
S1.004	28.441	249.5	s7	13.455	11.206	1.799	Open Manhole	1500
S1.005	22.265	69.4	S8	12.851	10.885	1.516	Open Manhole	1500
S1.006	13.083	34.2	S9	12.490	10.503	1.537	Open Manhole	1500
S1.007	9.704	13.8	S10	12.184	9.800	1.934	Open Manhole	1500
S1.008	9.891	98.9	SHW1	10.500	9.700	0.350	Open Manhole	1500
	26.139			10.500	8.600		Open Manhole	
S1.010			SHW2	10.500	8.570		Open Manhole	
S1.011	15.582	30.0	S25	11.640	8.050	3.140	Open Manhole	0

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Area Summary for Storm 1

Pipe	PIMP	PIMP	PIMP	Gross	Imp.	Pipe Total
Number	Type	Name	(%)	Area (ha)	Area (ha)	(ha)
1.000	_	_	100	0.077	0.077	0.077
2.000	_	_	100	0.034	0.034	0.034
1.001	User	_	100	0.023	0.023	0.023
	User	_	100	0.015	0.015	0.038
	User	_	100	0.009	0.009	0.046
	User	_	100	0.003	0.003	0.049
	User	_	100	0.009	0.009	0.058
	User	_	100	0.029	0.029	0.087
	User	_	100	0.008	0.008	0.095
1.002	-	_	100	0.027	0.027	0.027
3.000	-	_	100	0.080	0.080	0.080
1.003	-	_	100	0.016	0.016	0.016
4.000	_	_	100	0.091	0.091	0.091
1.004	User	_	100	0.011	0.011	0.011
	User	-	100	0.005	0.005	0.016
	User	_	100	0.009	0.009	0.025
	User	_	100	0.003	0.003	0.028
	User	-	100	0.014	0.014	0.042
	User	-	100	0.012	0.012	0.055
	User	-	100	0.014	0.014	0.068
	User	-	100	0.015	0.015	0.083
1.005	-	-	100	0.005	0.005	0.005
1.006	-	-	100	0.015	0.015	0.015
1.007	-	-	100	0.039	0.039	0.039
1.008	-	-	100	0.020	0.020	0.020
1.009	-	-	100	0.208	0.208	0.208
1.010	-	-	100	0.000	0.000	0.000
1.011	-	-	100	0.000	0.000	0.000
				Total	Total	Total
				0.791	0.791	0.791

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Network Classifications for Storm 1

PN	USMH Name	Pipe Dia (mm)	Min Cover Depth (m)	Max Cover Depth (m)	Pipe Type	MH Dia (mm)	MH Width (mm)	MH Ring Depth (m)	MH Type
							, ,		
S1.000	S2	300	1.500	1.500	Unclassified	1200	0	1.500	Unclassified
S2.000	S2A	150	1.500	1.582	Unclassified	1200	0	1.582	Unclassified
S1.001	s3	300	1.500	1.530	Unclassified	1200	0	1.500	Unclassified
S1.002	S4	300	1.500	2.038	Unclassified	1200	0	1.500	Unclassified
S3.000	S11	300	1.502	2.038	Unclassified	1200	0	1.502	Unclassified
S1.003	S5	450	2.038	2.503	Unclassified	1500	0	2.038	Unclassified
S4.000	S12	225	1.500	2.503	Unclassified	1200	0	1.500	Unclassified
S1.004	S6	450	1.618	2.503	Unclassified	1500	0	2.503	Unclassified
S1.005	s7	450	1.516	1.799	Unclassified	1500	0	1.799	Unclassified
S1.006	S8	450	1.516	1.537	Unclassified	1500	0	1.516	Unclassified
S1.007	S9	450	1.537	1.934	Unclassified	1500	0	1.537	Unclassified
S1.008	S10	450	0.350	1.934	Unclassified	1500	0	1.934	Unclassified
S1.009	SHW1	450	0.350	1.450	Unclassified	1500	0	0.350	Unclassified
S1.010	SSW11	450	1.450	1.480	Unclassified	1500	0	1.450	Unclassified
S1.011	SHW2	450	1.480	3.140	Unclassified	1500	0	1.480	Unclassified

$\underline{ \text{Free Flowing Outfall Details for Storm 1} }$

Outfall	Outfall	C. Level	I. Level	Min	D,L	W
Pipe Number	Name	(m)	(m)	I. Level	(mm)	(mm)
				(m)		
S1.011	S25	11.640	8.050	0.000	0	0

$\underline{\text{Simulation Criteria for Storm 1}}$

Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow 0.000
Areal Reduction Factor	1.000	MADD Factor * 10m³/ha Storage 2.000
Hot Start (mins)	0	Inlet Coefficient 0.800
Hot Start Level (mm)	0	Flow per Person per Day (1/per/day) 0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins) 60
Foul Sewage per hectare (1/s)	0.000	Output Interval (mins) 1

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 1 Number of Storage Structures 1 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type Summer
Return Period (years)	30	Cv (Summer) 0.750
Region Engla	nd and Wales	Cv (Winter) 0.840
M5-60 (mm)	26.250 Storn	n Duration (mins) 30
Ratio R	0.400	

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Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 2.000
Hot Start Level (mm) 0 Inlet Coefficient 0.800
Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (1/per/day) 0.000
Foul Sewage per hectare (1/s) 0.000

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 1 Number of Storage Structures 1 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 26.250 Cv (Summer) 0.750 Region England and Wales Ratio R 0.400 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status

DVD Status

ON

Inertia Status

ON

Profile(s) Summer and Winter
Duration(s) (mins) 15, 30, 60, 120, 240, 360, 480, 960, 1440

Return Period(s) (years) 1, 30, 100
Climate Change (%) 0, 0, 40

											Water	Surcharged	Flooded
	US/MH			Return	${\tt Climate}$	First	(X)	First (Y)	First (Z)	Overflow	Level	Depth	Volume
PN	Name	s	torm	Period	Change	Surch	arge	Flood	Overflow	Act.	(m)	(m)	(m³)
S1.000	S2	15	Winter	1	+0%	100/15	Summer				12.990	-0.229	0.000
S2.000	S2A	15	Winter	1	+0%	100/15	Summer				13.006	-0.082	0.000
S1.001	s3	15	Winter	1	+0%	100/15	Summer				12.826	-0.195	0.000
S1.002	S4	15	Winter	1	+0%	100/15	Summer				12.559	-0.205	0.000
s3.000	S11	15	Winter	1	+0%	30/15	Summer				11.731	-0.198	0.000
S1.003	S5	15	Winter	1	+0%	30/15	Summer				11.596	-0.226	0.000
S4.000	S12	15	Winter	1	+0%						12.621	-0.164	0.000
S1.004	S6	15	Winter	1	+0%	30/15	Summer				11.543	-0.227	0.000
S1.005	s7	15	Winter	1	+0%	100/15	Summer				11.364	-0.292	0.000
S1.006	S8	15	Winter	1	+0%	100/15	Summer				11.037	-0.298	0.000
S1.007	S9	15	Winter	1	+0%	100/15	Summer				10.638	-0.315	0.000
S1.008	S10	15	Winter	1	+0%	30/15	Summer				10.035	-0.215	0.000
S1.009	SHW1	15	Winter	1	+0%	100/360	Winter				9.843	-0.307	0.000
S1.010	SSW11	360	Winter	1	+0%	1/60	Winter				9.180	0.130	0.000
S1.011	SHW2	360	Winter	1	+0%	1/30	Winter				9.199	0.179	0.000

				Half Drain	Pipe		
	US/MH	Flow /	Overflow	Time	Flow		Level
P	N Name	Cap.	(1/s)	(mins)	(1/s)	Status	Exceeded
S1.	000 S2	0.13			14.3	OK	
S2.	000 S2A	0.42			6.3	OK	
S1.	001 S3	0.26			35.4	OK	
S1.	002 S4	0.21			39.7	OK	
S3.	000 S11	0.25			14.6	OK	
S1.	003 S5	0.43			56.3	OK	
S4.	000 S12	0.16			16.9	OK	
S1.	004 S6	0.48			84.0	OK	
S1.	005 S7	0.27			85.4	OK	
S1.	006 S8	0.25			87.5	OK	
S1.	007 S9	0.20			92.7	OK	
S1.	008 S10	0.53			95.0	OK	
			- 1 0 0 0				

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Innovyze	Network 2020.1	'

				Half Drain	Pipe		
	US/MH	Flow /	Overflow	Time	Flow		Level
PN	Name	Cap.	(1/s)	(mins)	(1/s)	Status	Exceeded
S1.009	SHW1	0.22			123.8	OK	
S1.010	SSW11	0.04			5.9	SURCHARGED	
S1.011	SHW2	0.01			3.2	SURCHARGED	

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SE1 9EA		Micco
Date 26/11/2021	Designed by CS	Desipago
File 132592 PHASE 4.MDX	Checked by AC	pranage
Innovyze	Network 2020.1	,

Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 2.000
Hot Start Level (mm) 0 Inlet Coefficient 0.800
Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (1/per/day) 0.000
Foul Sewage per hectare (1/s) 0.000

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 1 Number of Storage Structures 1 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 26.250 Cv (Summer) 0.750 Region England and Wales Ratio R 0.400 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status

DVD Status

ON

Inertia Status

Profile(s) Summer and Winter
Duration(s) (mins) 15, 30, 60, 120, 240, 360, 480, 960, 1440
Return Period(s) (years) 1, 30, 100
Climate Change (%) 0, 0, 40

	US/MH				Climate	First		First (Y)	First (Z)	Overflow	Water Level	Surcharged Depth	Volume
PN	Name	S	torm	Period	Change	Surch	arge	Flood	Overflow	Act.	(m)	(m)	(m³)
S1.000	S2	15	Winter	30	+0%	100/15	Summer				13.035	-0.184	0.000
S2.000	S2A	15	Winter	30	+0%	100/15	Summer				13.080	-0.008	0.000
S1.001	s3	15	Winter	30	+0%	100/15	Summer				12.911	-0.110	0.000
S1.002	S4	15	Winter	30	+0%	100/15	Summer				12.631	-0.133	0.000
s3.000	S11	15	Winter	30	+0%	30/15	Summer				11.962	0.033	0.000
S1.003	S5	15	Winter	30	+0%	30/15	Summer				11.884	0.062	0.000
S4.000	S12	15	Winter	30	+0%						12.659	-0.126	0.000
S1.004	S6	15	Winter	30	+0%	30/15	Summer				11.816	0.046	0.000
S1.005	s7	15	Winter	30	+0%	100/15	Summer				11.483	-0.173	0.000
S1.006	S8	15	Winter	30	+0%	100/15	Summer				11.150	-0.185	0.000
S1.007	S9	15	Winter	30	+0%	100/15	Summer				10.731	-0.222	0.000
S1.008	S10	15	Winter	30	+0%	30/15	Summer				10.325	0.075	0.000
S1.009	SHW1	15	Winter	30	+0%	100/360	Winter				9.946	-0.204	0.000
S1.010	SSW11	480	Winter	30	+0%	1/60	Winter				9.678	0.628	0.000
S1.011	SHW2	480	Winter	30	+0%	1/30	Winter				9.697	0.677	0.000

					Half Drain	Pipe		
		US/MH	Flow /	Overflow	Time	Flow		Level
	PN	Name	Cap.	(1/s)	(mins)	(1/s)	Status	Exceeded
	S1.000	S2	0.31			35.0	OK	
	S2.000	S2A	1.00			15.0	OK	
	S1.001	s3	0.71			95.9	OK	
	S1.002	S4	0.59			108.6	OK	
	S3.000	S11	0.58			34.0	SURCHARGED	
	S1.003	S5	1.12			144.8	SURCHARGED	
	S4.000	S12	0.40			41.4	OK	
	S1.004	S6	1.24			215.8	SURCHARGED	
	S1.005	s7	0.68			218.4	OK	
	S1.006	S8	0.64			224.7	OK	
	S1.007	S9	0.50			237.4	OK	
	S1.008	S10	1.37			244.1	SURCHARGED	
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				Half Drain	Pipe		
	US/MH	Flow /	Overflow	Time	Flow		Level
PN	Name	Cap.	(1/s)	(mins)	(1/s)	Status	Exceeded
S1.009	SHW1	0.55			310.8	OK	
S1.010	SSW11	0.04			6.1	SURCHARGED	
S1.011	SHW2	0.01			3.2	SURCHARGED	

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Innovyze	Network 2020.1	

Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 2.000
Hot Start Level (mm) 0 Inlet Coefficient 0.800
Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (1/per/day) 0.000
Foul Sewage per hectare (1/s) 0.000

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 1 Number of Storage Structures 1 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 26.250 Cv (Summer) 0.750 Region England and Wales Ratio R 0.400 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status

DVD Status

ON

Inertia Status

ON

Profile(s) Summer and Winter
Duration(s) (mins) 15, 30, 60, 120, 240, 360, 480, 960, 1440
Return Period(s) (years) 1, 30, 100
Climate Change (%) 0, 0, 40

											Water	Surcharged	Flooded
	US/MH			Return	${\tt Climate}$	First	(X)	First (Y)	First (Z)	Overflow	Level	Depth	Volume
PN	Name	s	torm	Period	Change	Surch	arge	Flood	Overflow	Act.	(m)	(m)	(m³)
S1.000	S2	15	Winter	100	+40%	100/15	Summer				13.784	0.565	0.000
S2.000	S2A	15	Winter	100	+40%	100/15	Summer				13.824	0.736	0.000
S1.001	s3	15	Winter	100	+40%	100/15	Summer				13.646	0.625	0.000
S1.002	S4	15	Winter	100	+40%	100/15	Summer				13.288	0.524	0.000
s3.000	S11	15	Winter	100	+40%	30/15	Summer				12.625	0.696	0.000
S1.003	S5	15	Winter	100	+40%	30/15	Summer				12.521	0.699	0.000
S4.000	S12	15	Winter	100	+40%						12.769	-0.016	0.000
S1.004	S 6	15	Winter	100	+40%	30/15	Summer				12.352	0.582	0.000
S1.005	s7	15	Winter	100	+40%	100/15	Summer				11.875	0.219	0.000
S1.006	S8	15	Winter	100	+40%	100/15	Summer				11.492	0.157	0.000
S1.007	S9	15	Winter	100	+40%	100/15	Summer				11.084	0.131	0.000
S1.008	S10	15	Winter	100	+40%	30/15	Summer				10.623	0.373	0.000
S1.009	SHW1	960	Winter	100	+40%	100/360	Winter				10.224	0.074	0.000
S1.010	SSW11	960	Winter	100	+40%	1/60	Winter				10.222	1.172	0.000
S1.011	SHW2	960	Winter	100	+40%	1/30	Winter				10.241	1.221	0.000

				Half Drain	Pipe		
	US/MH	Flow /	Overflow	Time	Flow		Level
PN	Name	Cap.	(1/s)	(mins)	(1/s)	Status	Exceeded
S1.000	S2	0.51			57.3	SURCHARGED	
S2.000	S2A	1.82			27.4	SURCHARGED	
S1.001	s3	1.11			149.5	SURCHARGED	
S1.002	S4	0.90			167.7	SURCHARGED	
s3.000	S11	1.01			59.8	SURCHARGED	
S1.003	S5	1.81			233.8	SURCHARGED	
S4.000	S12	0.72			75.6	OK	
S1.004	S6	2.06			358.9	SURCHARGED	
S1.005	s7	1.12			357.5	SURCHARGED	
S1.006	S8	1.04			360.8	SURCHARGED	
S1.007	S9	0.81			380.5	SURCHARGED	
S1.008	S10	2.19			390.6	SURCHARGED	

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				Half Drain	Pipe		
	US/MH	Flow /	Overflow	Time	Flow		Level
PN	Name	Cap.	(1/s)	(mins)	(1/s)	Status	Exceeded
S1.009	SHW1	0.07			38.9	FLOOD RISK	
S1.010	SSW11	0.05			6.2	FLOOD RISK	
S1.011	SHW2	0.01			3.8	FLOOD RISK	

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STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm 2

Pipe Sizes FH-PIPE Manhole Sizes FH-MH

FSR Rainfall Model - England and Wales

Return Period (years) 30 PIMP (%) 100

M5-60 (mm) 26.250 Add Flow / Climate Change (%) 0

Ratio R 0.400 Minimum Backdrop Height (m) 0.200

Maximum Rainfall (mm/hr) 50 Maximum Backdrop Height (m) 1.500

Maximum Time of Concentration (mins) 30 Min Design Depth for Optimisation (m) 1.200

Foul Sewage (1/s/ha) 0.000 Min Vel for Auto Design only (m/s) 1.00

Volumetric Runoff Coeff. 0.750 Min Slope for Optimisation (1:X) 500

Designed with Level Soffits

Time Area Diagram for Storm 2

				Time	
(mins)	(ha)	(mins)	(ha)	(mins)	(ha)
0-4	0.169	4-8	0.275	8-12	0.000

Total Area Contributing (ha) = 0.445

Total Pipe Volume $(m^3) = 15.488$

Network Design Table for Storm 2

« - Indicates pipe capacity < flow</pre>

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)		Base Flow (1/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
S1.000	24.482	0.109	224.6	0.061	5.00	0.0	0.600	0	300	Pipe/Conduit	3
S2.000	18.659	0.537	34.7	0.062	5.00	0.0	0.600	0	225	Pipe/Conduit	8
S1.001	33.745	0.279	120.9	0.081	0.00	0.0	0.600	0	300	Pipe/Conduit	₽
S1.002	21.836	0.829	26.3	0.060	0.00	0.0	0.600	0	300	Pipe/Conduit	ef®°
S1.003	13.013	0.304	42.8	0.068	0.00	0.0	0.600	0	300	Pipe/Conduit	eg [®]
S1.004	10.698	0.040	267.5	0.113	0.00	0.0	0.600	0	300	Pipe/Conduit	eg [®]
S1.005	9.235	0.011	839.5	0.000	0.00	0.0	0.600	0	375	Pipe/Conduit	₩
S1.006	46.746	0.036	1298.5	0.000	0.00	0.0	0.600	0	375	Pipe/Conduit	&
S1.007	7.723	1.500	5.1	0.000	0.00	0.0	0.600	0	450	Pipe/Conduit	ø

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (1/s)	Foul (1/s)	Add Flow (1/s)	Vel (m/s)	Cap (1/s)	Flow (1/s)
S1.000	50.00	5.39	11.968	0.061	0.0	0.0	0.0	1.04	73.9	8.3
S2.000	50.00	5.14	12.471	0.062	0.0	0.0	0.0	2.23	88.5	8.4
\$1.001 \$1.002 \$1.003 \$1.004 \$1.005 \$1.006 \$1.007	50.00 50.00 50.00 50.00 50.00 50.00	5.90 5.99 6.18 6.43 8.00	11.859 11.580 10.751 10.447 10.332 10.336	0.204 0.263 0.332 0.445 0.445 0.445	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	1.43 3.08 2.41 0.96 0.62 0.49	101.0 217.4 170.3 67.6 68.2 54.6«	27.6 35.7 44.9 60.3 60.3 60.3

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Manhole Schedules for Storm 2

					Hamiot	<u> </u>	duics for	. DCOIM 2	=			
MH Name	MH CL (m)	MH Depth (m)	Coni	MH nection	MH Diam.,L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Backdrop (mm)
s13	13.768	1.800	Open	Manhole	1200	S1.000	11.968	300				
S18	14.196	1.725	Open	Manhole	1200	s2.000	12.471	225				
S14	13.943	2.084	Open	Manhole	1200	s1.001	11.859	300	S1.000	11.859	300	
									s2.000	11.934	225	
S15	13.097	1.517	Open	Manhole	1200	s1.002	11.580	300	S1.001	11.580	300	
S16	12.551	1.800	Open	Manhole	1200	s1.003	10.751	300	S1.002	10.751	300	
S17	12.247	1.800	Open	Manhole	1200	S1.004	10.447	300	S1.003	10.447	300	
SHW2	12.225	1.893	Open	Manhole	1500	s1.005	10.332	375	S1.004	10.407	300	
SSW14	11.400	1.079	Open	Manhole	1500	s1.006	10.336	375	S1.005	10.321	375	
SHW3	11.400	1.100	Open	Manhole	1500	s1.007	10.300	450	S1.006	10.300	375	
S26	10.600	1.800	Open	Manhole	0		OUTFALL		S1.007	8.800	450	

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S13	617140.012	166732.189	617140.012	166732.189	Required	4
S18	617150.297	166774.081	617150.297	166774.081	Required	9
S14	617145.546	166756.038	617145.546	166756.038	Required	-
S15	617178.780	166750.192	617178.780	166750.192	Required	
S16	617200.246	166746.186	617200.246	166746.186	Required	
s17	617212.865	166749.363	617212.865	166749.363	Required	0
SHW2	617222.402	166754.211	617222.402	166754.211	Required	.0.
SSW14	617230.682	166750.122	617230.682	166750.122	Required	
SHW3	617230.185	166703.379	617230.185	166703.379	Required	
S26	617229.665	166695.673			No Entry	

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PIPELINE SCHEDULES for Storm 2

Upstream Manhole

PN	Hyd Sect	Diam (mm)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.000	0	300	s13	13.768	11.968	1.500	Open Manhole	1200
s2.000	0	225	S18	14.196	12.471	1.500	Open Manhole	1200
S1.001	0	300	S14	13.943	11.859	1.784	Open Manhole	1200
S1.002	0	300	S15	13.097	11.580	1.217	Open Manhole	1200
S1.003	0	300	S16	12.551	10.751	1.500	Open Manhole	1200
S1.004	0	300	S17	12.247	10.447	1.500	Open Manhole	1200
S1.005	0	375	SHW2	12.225	10.332	1.518	Open Manhole	1500
S1.006	0	375	SSW14	11.400	10.336	0.689	Open Manhole	1500
S1.007	0	450	SHW3	11.400	10.300	0.650	Open Manhole	1500

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.000	24.482	224.6	S14	13.943	11.859	1.784	Open Manhole	1200
s2.000	18.659	34.7	S14	13.943	11.934	1.784	Open Manhole	1200
S1.001	33.745	120.9	S15	13.097	11.580	1.217	Open Manhole	1200
S1.002	21.836	26.3	S16	12.551	10.751	1.500	Open Manhole	1200
S1.003	13.013	42.8	S17	12.247	10.447	1.500	Open Manhole	1200
S1.004	10.698	267.5	SHW2	12.225	10.407	1.518	Open Manhole	1500
S1.005	9.235	839.5	SSW14	11.400	10.321	0.704	Open Manhole	1500
S1.006	46.746	1298.5	SHW3	11.400	10.300	0.725	Open Manhole	1500
S1.007	7.723	5.1	S26	10.600	8.800	1.350	Open Manhole	0

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Area Summary for Storm 2

Pipe	PIMP	PIMP	PIMP	Gross	Imp.	Pipe Total
Number	Туре	Name	(%)	Area (ha)	Area (ha)	(ha)
1.000	_	_	100	0.061	0.061	0.061
2.000	_	_	100	0.062	0.062	0.062
1.001	User	_	100	0.009	0.009	0.009
	User	_	100	0.014	0.014	0.023
	User	_	100	0.003	0.003	0.025
	User	_	100	0.007	0.007	0.033
	User	_	100	0.003	0.003	0.036
	User	_	100	0.030	0.030	0.066
	User	_	100	0.015	0.015	0.081
1.002	User	-	100	0.015	0.015	0.015
	User	_	100	0.014	0.014	0.029
	User	-	100	0.031	0.031	0.060
1.003	User	-	100	0.037	0.037	0.037
	User	-	100	0.011	0.011	0.048
	User	_	100	0.005	0.005	0.053
	User	_	100	0.015	0.015	0.068
1.004	User	_	100	0.015	0.015	0.015
	User	-	100	0.014	0.014	0.029
	User	_	100	0.014	0.014	0.043
	User	_	100	0.013	0.013	0.056
	User	-	100	0.020	0.020	0.076
	User	_	100	0.037	0.037	0.113
1.005	_	_	100	0.000	0.000	0.000
1.006	-	-	100	0.000	0.000	0.000
1.007	_	_	100	0.000	0.000	0.000
				Total	Total	Total
				0.445	0.445	0.445

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Network Classifications for Storm 2

PN	USMH Name	Pipe Dia (mm)	Min Cover Depth (m)	Max Cover Depth (m)	Pipe Type	MH Dia (mm)	MH Width (mm)	MH Ring Depth (m)	MH Type
S1.000	S13	300	1.500	1.784	Unclassified	1200	0	1.500	Unclassified
S2.000	S18	225	1.500	1.784	Unclassified	1200	0	1.500	Unclassified
S1.001	S14	300	1.217	1.784	Unclassified	1200	0	1.784	Unclassified
S1.002	S15	300	1.217	1.500	Unclassified	1200	0	1.217	Unclassified
S1.003	S16	300	1.500	1.526	Unclassified	1200	0	1.500	Unclassified
S1.004	S17	300	1.438	1.518	Unclassified	1200	0	1.500	Unclassified
S1.005	SHW2	375	0.704	1.518	Unclassified	1500	0	1.518	Unclassified
S1.006	SSW14	375	0.689	0.725	Unclassified	1500	0	0.689	Unclassified
S1.007	SHW3	450	0.650	1.350	Unclassified	1500	0	0.650	Unclassified

Free Flowing Outfall Details for Storm 2

Outfall Pipe Number		C. Level (m)		Min I. Level (m)	•	
S1.007	S26	10.600	8.800	0.000	0	0

Simulation Criteria for Storm 2

Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow 0.000
Areal Reduction Factor	1.000	MADD Factor * 10m3/ha Storage 2.000
Hot Start (mins)	0	Inlet Coefficient 0.800
Hot Start Level (mm)	0	Flow per Person per Day (1/per/day) 0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins) 60
Foul Sewage per hectare (1/s)	0.000	Output Interval (mins) 1

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 1 Number of Storage Structures 1 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type	Summer
Return Period (years)	30	Cv (Summer)	0.750
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	26.250	Storm Duration (mins)	30
Ratio R	0 400		

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$\underline{\hbox{Simulation Criteria}}$

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 2.000
Hot Start Level (mm) 0 Inlet Coefficient 0.800
Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (1/per/day) 0.000
Foul Sewage per hectare (1/s) 0.000

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 1 Number of Storage Structures 1 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 26.250 Cv (Summer) 0.750 Region England and Wales Ratio R 0.400 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status

DVD Status

ON

Inertia Status

ON

									Water	Surcharged	Flooded
	US/MH		Return	Climate	First (X)	First (Y)	First (Z)	Overflow	Level	Depth	Volume
PN	Name	Storm	Period	Change	Surcharge	Flood	Overflow	Act.	(m)	(m)	(m³)
S1.000	S13	15 Winter	1	1.00	100/15 Summer	100/15 Summer			12.052	-0.216	0.000
						100/15 Summer					
S2.000	S18	15 Winter	1	+0%	100/15 Summer				12.528	-0.168	0.000
S1.001	S14	15 Winter	1	+0%	30/15 Winter				11.988	-0.171	0.000
S1.002	S15	15 Winter	1	+0%	30/15 Summer	100/15 Summer			11.678	-0.202	0.000
S1.003	S16	15 Winter	1	+0%	30/15 Summer	100/15 Summer			10.882	-0.169	0.000
S1.004	S17	15 Winter	1	+0%	1/15 Summer				10.796	0.049	0.000
S1.005	SHW2	15 Summer	1	+0%	1/15 Summer				10.716	0.009	0.000
S1.006	SSW14	480 Winter	1	+0%	30/120 Winter				10.565	-0.146	0.000
S1.007	SHW3	480 Winter	1	+0%	30/120 Winter	100/240 Winter			10.564	-0.186	0.000

				Half Drain	Pipe		
	US/MH	Flow /	Overflow	Time	Flow		Level
PN	Name	Cap.	(1/s)	(mins)	(1/s)	Status	Exceeded
S1.000	S13	0.17			11.1	OK	2
S2.000	S18	0.14			11.5	OK	
S1.001	S14	0.38			35.2	OK	
S1.002	S15	0.23			44.4	OK	3
S1.003	S16	0.40			54.9	OK	3
S1.004	S17	1.35			72.5	SURCHARGED	
S1.005	SHW2	1.26			68.0	SURCHARGED	
S1.006	SSW14	0.05			2.6	OK	
	SSWIA	0.05			2.0	NO	
S1.007	SHW3	0.00			1.7	OK	1

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Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 2.000
Hot Start Level (mm) 0 Inlet Coefficient 0.800
Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (1/per/day) 0.000
Foul Sewage per hectare (1/s) 0.000

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 1 Number of Storage Structures 1 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 26.250 Cv (Summer) 0.750 Region England and Wales Ratio R 0.400 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status

DVD Status

ON

Inertia Status

PN	US/MH Name	Storm		Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow	Overflow Act.	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)
S1.000	S13	15 Winter	30	+0%	100/15 Summer	100/15 Summer			12.224	-0.044	0.000
S2.000	S18	15 Winter	30	+0%	100/15 Summer				12.564	-0.132	0.000
S1.001	S14	15 Winter	30	+0%	30/15 Winter				12.171	0.012	0.000
S1.002	S15	15 Winter	30	+0%	30/15 Summer	100/15 Summer			11.945	0.065	0.000
S1.003	S16	15 Winter	30	+0%	30/15 Summer	100/15 Summer			11.687	0.636	0.000
S1.004	S17	15 Winter	30	+0%	1/15 Summer				11.415	0.668	0.000
S1.005	SHW2	15 Winter	30	+0%	1/15 Summer				10.890	0.183	0.000
S1.006	SSW14	960 Winter	30	+0%	30/120 Winter				10.841	0.130	0.000
S1.007	SHW3	960 Winter	30	+0%	30/120 Winter	100/240 Winter			10.958	0.208	0.000

				Half Drain	Pipe		
	US/MH	Flow /	Overflow	Time	Flow		Level
PN	Name	Cap.	(1/s)	(mins)	(1/s)	Status	Exceeded
S1.000	S13	0.40			26.3	OK	2
S2.000	S18	0.35			28.2	OK	
S1.001	S14	0.94			87.4	SURCHARGED	
S1.002	S15	0.59			111.9	SURCHARGED	3
S1.003	S16	0.98			134.7	SURCHARGED	3
S1.004	S17	3.34			179.2	SURCHARGED	
S1.005	SHW2	3.33			179.2	SURCHARGED	
S1.006	SSW14	0.22			12.1	SURCHARGED	
S1.007	SHW3	0.00			1.7		1

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Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 2.000
Hot Start Level (mm) 0 Inlet Coefficient 0.800
Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (1/per/day) 0.000
Foul Sewage per hectare (1/s) 0.000

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 1 Number of Storage Structures 1 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 26.250 Cv (Summer) 0.750 Region England and Wales Ratio R 0.400 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status

DVD Status

ON

Inertia Status

PN	US/MH Name	St	orm		Climate Change	First (Surchar		First Flo		First (Z)	Water Level (m)	Surcharged Depth (m)
S1.000	S13	15	Winter	100	+40%	100/15 Su	ummer	100/15	Summer		13.770	1.502
S2.000	S18	15	Winter	100	+40%	100/15 Su	ummer				13.952	1.256
S1.001	S14	15	Winter	100	+40%	30/15 Wi	nter				13.753	1.594
S1.002	S15	15	Winter	100	+40%	30/15 Su	ummer	100/15	Summer		13.100	1.220
S1.003	S16	15	Winter	100	+40%	30/15 Su	ummer	100/15	Summer		12.557	1.506
S1.004	S17	15	Summer	100	+40%	1/15 Su	ummer				12.179	1.432
S1.005	SHW2	1440	Winter	100	+40%	1/15 Su	ummer				11.212	0.505
S1.006	SSW14	1440	Winter	100	+40%	30/120 Wi	nter				11.212	0.501
S1.007	SHW3	240	Winter	100	+40%	30/120 Wi	nter	100/240	Winter		11.400	0.650

		Flooded			Half Drain	Pipe		
	US/MH	Volume	Flow /	Overflow	Time	Flow		Level
PN	Name	(m³)	Cap.	(1/s)	(mins)	(1/s)	Status	Exceeded
S1.000	S13	1.668	0.68			44.5	FLOOD	2.
S2.000	S18	0.000	0.61				FLOOD RISK	_
S1.001	S14	0.000	1.46			135.6	FLOOD RISK	
S1.002	S15	3.091	0.81			154.7	FLOOD	3
S1.003	S16	6.349	1.50			207.3	FLOOD	3
S1.004	S17	0.000	4.71			252.4	FLOOD RISK	
S1.005	SHW2	0.000	0.30			15.9	SURCHARGED	
S1.006	SSW14	0.000	0.31			17.4	FLOOD RISK	
S1.007	SHW3	0.136	0.00			1.7	FLOOD	1

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STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm 3

Pipe Sizes FH-PIPE Manhole Sizes FH-MH

FSR Rainfall Model - England and Wales

Return Period (years) 100 PIMP (%) 100 M5-60 (mm) 26.250 Add Flow / Climate Change (%) 0 Ratio R 0.400 Minimum Backdrop Height (m) 0.200 Maximum Rainfall (mm/hr) 50 Maximum Backdrop Height (m) 1.500 Maximum Time of Concentration (mins) 30 Min Design Depth for Optimisation (m) 1.200 Foul Sewage (1/s/ha) 0.000 Min Vel for Auto Design only (1/s/ha) 1.00 Volumetric Runoff Coeff. 0.750 Min Slope for Optimisation (1:X) 500

Designed with Level Soffits

Time Area Diagram for Storm 3

Time Area (mins) (ha) (mins) (ha) (ha) 0-4 0.292 4-8 0.124

Total Area Contributing (ha) = 0.416

Total Pipe Volume $(m^3) = 14.518$

Network Design Table for Storm 3

PN	Length	Fall	Slope	I.Area	T.E.	Ba	ıse	k	HYD	DIA	Section Type	Auto
	(m)	(m)	(1:X)	(ha)	(mins)	Flow	(1/s)	(mm)	SECT	(mm)		Design
S1.000	25.910	1.200	21.6	0.038	5.00		0.0	0.600	0	150	Pipe/Conduit	8
S1.001	21.216	0.305	69.6	0.056	0.00		0.0	0.600	0	225	Pipe/Conduit	ag [®]
s2.000	12.848	0.099	129.8	0.009	5.00		0.0	0.600	0	150	Pipe/Conduit	₿
S1.002	21.765	0.539	40.4	0.057	0.00		0.0	0.600	0	225	Pipe/Conduit	ø°
S1.003	16.083	0.879	18.3	0.048	0.00		0.0	0.600	0	300	Pipe/Conduit	e e ®
s3.000	25.671	0.166	154.6	0.030	5.00		0.0	0.600	0	225	Pipe/Conduit	8
S1.004	29.393	0.912	32.2	0.050	0.00		0.0	0.600	0	300	Pipe/Conduit	eF
S1.005	20.460	0.058	352.8	0.128	0.00		0.0	0.600	0	450	Pipe/Conduit	E

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (1/s)		Add Flow (1/s)	Vel (m/s)	Cap (1/s)	Flow (1/s)
S1.000 S1.001	50.00		12.275 11.000	0.038 0.094	0.0	0.0	0.0	2.18 1.57	38.5 62.4	5.1 12.7
S2.000	50.00	5.24	11.008	0.009	0.0	0.0	0.0	0.88	15.6	1.2
S1.002 S1.003	50.00		10.695 10.081	0.160 0.208	0.0	0.0	0.0	2.06	82.1 261.0	21.7 28.2
s3.000	50.00	5.41	9.518	0.030	0.0	0.0	0.0	1.05	41.7	4.1
S1.004 S1.005	50.00	5.85 6.16	9.202 8.140	0.288 0.416	0.0	0.0	0.0		196.5 171.2	39.0 56.3

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Network Design Table for Storm 3

PN	Length	Fall	Slope	I.Area	T.E.	Base		k	HYD	DIA	Section Type	Auto
	(m)	(m)	(1:X)	(ha)	(mins)	Flow (1/	's)	(mm)	SECT	(mm)		Design
S1.00	6 6.802	0.019	358.0	0.000	0.00	(0.0	0.600	0	450	Pipe/Conduit	18
S1.00	7 18.850	0.053	355.7	0.000	0.00	(0.0	0.600	0	450	Pipe/Conduit	
S1.00	3.493	0.010	349.3	0.000	0.00	(0.0	0.600	0	450	Pipe/Conduit	of of

Network Results Table

PN	Rain	T.C.	US/IL	Σ I.Area	ΣΕ	Base	Foul	Add Flow	Vel	Cap	Flow
	(mm/hr)	(mins)	(m)	(ha)	Flow	(1/s)	(1/s)	(1/s)	(m/s)	(1/s)	(1/s)
S1.0	06 50.00	6.27	8.082	0.416		0.0	0.0	0.0	1.07	170.0	56.3
S1.0	07 50.00	6.56	8.063	0.416		0.0	0.0	0.0	1.07	170.5	56.3
S1.0	08 50.00	6.62	8.010	0.416		0.0	0.0	0.0	1.08	172.1	56.3

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Manhole Schedules for Storm 3

MH Name	MH CL (m)	MH Depth		MH ection	MH Diam.,L*W	PN	Pipe Out Invert	Diameter	PN	Pipes In Invert	Diameter	Backdrop
Name	CL (III)	(m)	Com	lection	(mm)	PN	Level (m)	(mm)	PN	Level (m)	(mm)	(mm)
S19	13.925	1.650	Open	Manhole	1200	S1.000	12.275	150				
S20	13.176	2.176	Open	Manhole	1200	S1.001	11.000	225	S1.000	11.075	150	
S31	12.658	1.650	Open	Manhole	1200	S2.000	11.008	150				
S21	12.666	1.971	Open	Manhole	1200	S1.002	10.695	225	S1.001	10.695	225	
									S2.000	10.909	150	139
S22	11.881	1.800	Open	Manhole	1200	S1.003	10.081	300	S1.002	10.156	225	
S23A	10.668	1.150	Open	Manhole	1200	s3.000	9.518	225				
S23	11.401	2.199	Open	Manhole	1200	S1.004	9.202	300	s1.003	9.202	300	
									s3.000	9.352	225	75
S24	10.800	2.660	Open	Manhole	1500	S1.005	8.140	450	S1.004	8.290	300	
S27	10.300	2.218	Open	Manhole	1500	S1.006	8.082	450	S1.005	8.082	450	
S28	10.300	2.237	Open	Manhole	1500	S1.007	8.063	450	S1.006	8.063	450	
S30	9.500	1.490	Open	Manhole	1500	S1.008	8.010	450	S1.007	8.010	450	
SHW5	9.400	1.400	Open	Manhole	0		OUTFALL		S1.008	8.000	450	

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
S19	617184.958	166796.765	617184.958	166796.765	Required	0-
S20	617210.526	166792.571	617210.526	166792.571	Required	
S31	617230.209	166779.404	617230.209	166779.404	Required	
S21	617231.738	166792.161	617231.738	166792.161	Required	
S22	617253.213	166795.698	617253.213	166795.698	Required	
S23A	617291.979	166811.183	617291.979	166811.183	Required	
S23	617268.305	166801.256	617268.305	166801.256	Required	-9-
S24	617279.394	166774.036	617279.394	166774.036	Required	,
S27	617287.225	166755.133	617287.225	166755.133	Required	6
S28	617293.435	166752.357	617293.435	166752.357	Required	-
S30	617310.813	166759.661	617310.813	166759.661	Required	.0.

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Manhole Schedules for Storm 3

MH	Manhole	Manhole	Intersection	Intersection	Manhole	Layout
Name	Easting	Northing	Easting	Northing	Access	(North)
	(m)	(m)	(m)	(m)		

SHW5 617313.895 166758.017

No Entry

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PIPELINE SCHEDULES for Storm 3

Upstream Manhole

PN	-	Diam (mm)		C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
S1.000	0	150	S19	13.925	12.275		Open Manhole	
S1.001	0	225	S20	13.176	11.000	1.951	Open Manhole	1200
S2.000	0	150	S31	12.658	11.008	1.500	Open Manhole	1200
S1.002	0	225	S21	12.666	10.695	1.746	Open Manhole	1200
S1.003	0	300	S22	11.881	10.081	1.500	Open Manhole	1200
s3.000	0	225	S23A	10.668	9.518	0.925	Open Manhole	1200
S1.004	0	300	S23	11.401	9.202	1.899	Open Manhole	1200
S1.005	0	450	S24	10.800	8.140	2.210	Open Manhole	1500
S1.006	0	450	S27	10.300	8.082	1.768	Open Manhole	1500
S1.007	0	450	S28	10.300	8.063	1.787	Open Manhole	1500
S1.008	0	450	S30	9.500	8.010	1.040	Open Manhole	1500

Downstream Manhole

PN	Length	Slope	MH	C.Level	I.Level	D.Depth	MH	MH DIAM., L*W
	(m)	(1:X)	Name	(m)	(m)	(m)	Connection	(mm)
S1.000	25.910	21.6	S20	13.176	11.075	1.951	Open Manhole	1200
S1.001	21.216	69.6	S21	12.666			Open Manhole	
S2.000	12.848	129.8	S21	12.666	10.909	1.607	Open Manhole	1200
S1.002	21.765	40.4	S22	11.881	10.156	1.500	Open Manhole	1200
S1.003	16.083	18.3	S23	11.401	9.202	1.899	Open Manhole	1200
s3.000	25.671	154.6	S23	11.401	9.352	1.824	Open Manhole	1200
S1.004	29.393	32.2	S24	10.800	8.290	2.210	Open Manhole	1500
S1.005	20.460	352.8	S27	10.300	8.082	1.768	Open Manhole	1500
S1.006	6.802	358.0	S28	10.300	8.063	1.787	Open Manhole	1500
S1.007	18.850	355.7	S30	9.500	8.010	1.040	Open Manhole	1500
S1.008	3.493	349.3	SHW5	9.400	8.000	0.950	Open Manhole	0

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Area Summary for Storm 3

Pipe Number		PIMP Name	PIMP (%)	Gross Area (ha)	Imp. Area (ha)	Pipe Total (ha)
1.000	_	_	100	0.038	0.038	0.038
1.001	_	_	100	0.056	0.056	0.056
2.000	_	_	100	0.009	0.009	0.009
1.002	_	_	100	0.057	0.057	0.057
1.003	_	_	100	0.048	0.048	0.048
3.000	_	_	100	0.030	0.030	0.030
1.004	_	_	100	0.050	0.050	0.050
1.005	_	_	100	0.128	0.128	0.128
1.006	_	_	100	0.000	0.000	0.000
1.007	_	_	100	0.000	0.000	0.000
1.008	_	_	100	0.000	0.000	0.000
				Total	Total	Total
				0.416	0.416	0.416

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Network Classifications for Storm 3

PN	USMH Name	Pipe Dia (mm)	Min Cover Depth (m)	Max Cover Depth (m)	Pipe Type	MH Dia (mm)	MH Width (mm)	MH Ring Depth (m)	МН Туре
S1.000	S19	150	1.500	1.951	Unclassified	1200	0	1.500	Unclassified
S1.001	S20	225	1.746	1.979	Unclassified	1200	0	1.951	Unclassified
S2.000	S31	150	1.440	1.607	Unclassified	1200	0	1.500	Unclassified
S1.002	S21	225	1.500	1.746	Unclassified	1200	0	1.746	Unclassified
S1.003	S22	300	1.500	1.899	Unclassified	1200	0	1.500	Unclassified
s3.000	S23A	225	0.925	1.824	Unclassified	1200	0	0.925	Unclassified
S1.004	S23	300	1.892	2.210	Unclassified	1200	0	1.899	Unclassified
S1.005	S24	450	1.768	2.210	Unclassified	1500	0	2.210	Unclassified
S1.006	S27	450	1.768	1.787	Unclassified	1500	0	1.768	Unclassified
S1.007	S28	450	1.040	1.787	Unclassified	1500	0	1.787	Unclassified
S1.008	S30	450	0.950	1.040	Unclassified	1500	0	1.040	Unclassified

Free Flowing Outfall Details for Storm 3

Outfall	Outfall C.	Level I	. Level	Min	D,L	W
Pipe Number	Name	(m)	(m)	I. Level	(mm)	(mm)
				(m)		
S1.008	SHW5	9.400	8.000	0.000	0	0

Simulation Criteria for Storm 3

Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow C	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m³/ha Storage 2	2.000
Hot Start (mins)	0	Inlet Coefficcient C	0.800
Hot Start Level (mm)	0	Flow per Person per Day (1/per/day) 0	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (1/s)	0.000	Output Interval (mins)	1

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 0 Number of Storage Structures 0 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type	Summer
Return Period (years)	30	Cv (Summer)	0.750
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	26.250	Storm Duration (mins)	30
Ratio R	0.400		

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135 Park Street	HERNE BAY	
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SE1 9EA		Micco
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Innovyze	Network 2020.1	•

Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 2.000
Hot Start Level (mm) 0 Inlet Coefficient 0.800
Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (1/per/day) 0.000
Foul Sewage per hectare (1/s) 0.000

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 0 Number of Storage Structures 0 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 26.250 Cv (Summer) 0.750 Region England and Wales Ratio R 0.400 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status

DVD Status

ON

Inertia Status

ON

PN	US/MH Name	Storm		Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow	Overflow Act.	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)
S1.000	S19	15 Winte	r 1	+0%	100/15 Summe:	r			12.320	-0.105	0.000
S1.001	S20	15 Winte	r 1	+0%	100/15 Summe:	r			11.081	-0.144	0.000
S2.000	S31	15 Winte	r 1	+0%	100/15 Summe:	r			11.042	-0.116	0.000
S1.002	S21	15 Winte	r 1	+0%	100/15 Summe:	r			10.788	-0.132	0.000
S1.003	S22	15 Winte	r 1	+0%	100/15 Summe:	r			10.160	-0.221	0.000
S3.000	S23A	15 Winte	r 1	+0%	100/15 Summe:	r			9.575	-0.168	0.000
S1.004	S23	15 Winte	r 1	+0%	100/15 Summe:	r			9.308	-0.194	0.000
S1.005	S24	15 Winte	r 1	+0%	30/15 Summe:	r			8.391	-0.199	0.000
S1.006	S27	15 Winte	r 1	+0%	30/15 Summe:	r			8.348	-0.184	0.000
S1.007	S28	15 Winte	r 1	+0%	30/15 Summe:	r			8.297	-0.216	0.000
S1.008	S30	15 Winte	r 1	+0%	30/15 Summe:	r			8.248	-0.212	0.000

				Half Drain	Pipe		
	US/MH	Flow /	Overflow	Time	Flow		Level
PN	Name	Cap.	(1/s)	(mins)	(1/s)	Status	Exceeded
S1.000	S19	0.19			7.1	OK	
S1.001	S20	0.28			15.8	OK	
S2.000	S31	0.12			1.7	OK	
S1.002	S21	0.35			26.4	OK	
S1.003	S22	0.15			34.0	OK	
s3.000	S23A	0.14			5.5	OK	
S1.004	S23	0.27			47.5	OK	
S1.005	S24	0.48			66.9	OK	
S1.006	S27	0.65			66.3	OK	
S1.007	S28	0.48			66.0	OK	
S1.008	S30	0.54			65.6	OK	

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Innovyze	Network 2020.1			

Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 2.000
Hot Start Level (mm) 0 Inlet Coefficient 0.800
Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (1/per/day) 0.000
Foul Sewage per hectare (1/s) 0.000

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 0 Number of Storage Structures 0 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 26.250 Cv (Summer) 0.750 Region England and Wales Ratio R 0.400 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status

DVD Status

ON

Inertia Status

ON

PN	US/MH Name	Storm		Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow	Overflow Act.	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)
S1.000	S19	15 Winter	30	+0%	100/15 Summer				12.348	-0.077	0.000
S1.001	S20	15 Winter	30	+0%	100/15 Summer				11.151	-0.074	0.000
S2.000	S31	15 Winter	30	+0%	100/15 Summer				11.063	-0.095	0.000
S1.002	S21	15 Winter	30	+0%	100/15 Summer				10.906	-0.014	0.000
S1.003	S22	15 Winter	30	+0%	100/15 Summer				10.221	-0.160	0.000
S3.000	S23A	15 Winter	30	+0%	100/15 Summer				9.611	-0.132	0.000
S1.004	S23	15 Winter	30	+0%	100/15 Summer				9.400	-0.102	0.000
S1.005	S24	15 Winter	30	+0%	30/15 Summer				8.869	0.279	0.000
S1.006	S27	15 Winter	30	+0%	30/15 Summer				8.752	0.220	0.000
S1.007	S28	15 Winter	30	+0%	30/15 Summer				8.646	0.133	0.000
S1.008	S30	15 Winter	30	+0%	30/15 Summer				8.538	0.078	0.000

				Half Drain	Pipe		
	US/MH	Flow /	Overflow	Time	Flow		Level
PN	Name	Cap.	(1/s)	(mins)	(1/s)	Status	Exceeded
S1.000	S19	0.47			17.3	OK	
S1.000	S20	0.47			44.3	OK	
S2.000	S31	0.29			4.1	OK	
S1.002	S21	1.00			74.7	OK	
S1.003	S22	0.44			98.0	OK	
S3.000	S23A	0.35			13.4	OK	
S1.004	S23	0.76			135.0	OK	
S1.005	S24	1.35			187.1	SURCHARGED	
S1.006	S27	1.83			187.1	SURCHARGED	
S1.007	S28	1.35			183.8	SURCHARGED	
S1.008	S30	1.52			184.6	SURCHARGED	

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Innovyze	Network 2020.1	1

Simulation Criteria

Areal Reduction Factor 1.000 Additional Flow - % of Total Flow 0.000
Hot Start (mins) 0 MADD Factor * 10m³/ha Storage 2.000
Hot Start Level (mm) 0 Inlet Coefficient 0.800
Manhole Headloss Coeff (Global) 0.500 Flow per Person per Day (1/per/day) 0.000
Foul Sewage per hectare (1/s) 0.000

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 0 Number of Storage Structures 0 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 26.250 Cv (Summer) 0.750 Region England and Wales Ratio R 0.400 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status

DVD Status

ON

Inertia Status

ON

									Water	Surcharged	Flooded
	US/MH		Return	${\tt Climate}$	First (X)	First (Y)	First (Z)	Overflow	Level	Depth	Volume
PN	Name	Storm	Period	Change	Surcharge	Flood	Overflow	Act.	(m)	(m)	(m³)
S1.000	919	15 Winter	100	+102	100/15 Summe	r			12.947	0.522	0.000
\$1.000		15 Winter			100/15 Summe				12.347	1.140	0.000
S2.000	S31	15 Winter	100	+40%	100/15 Summe	r			12.010	0.852	0.000
S1.002	S21	15 Winter	100	+40%	100/15 Summe	r			11.993	1.073	0.000
S1.003	S22	15 Winter	100	+40%	100/15 Summe	r			10.897	0.516	0.000
s3.000	S23A	15 Winter	100	+40%	100/15 Summe	r			10.608	0.865	0.000
S1.004	S23	15 Winter	100	+40%	100/15 Summe	r			10.544	1.042	0.000
S1.005	S24	15 Winter	100	+40%	30/15 Summe	r			9.450	0.860	0.000
S1.006	S27	15 Winter	100	+40%	30/15 Summe	r			9.188	0.656	0.000
S1.007	S28	15 Winter	100	+40%	30/15 Summe	r			8.931	0.418	0.000
S1.008	S30	15 Winter	100	+40%	30/15 Summe	r			8.678	0.218	0.000

				Half Drain	Pipe		
	US/MH	Flow /	Overflow	Time	Flow		Level
PN	Name	Cap.	(1/s)	(mins)	(1/s)	Status	Exceeded
S1.000	S19	0.76			27.0	SURCHARGED	
S1.001	S20	1.14			64.7	SURCHARGED	
S2.000	S31	0.83			11.8	SURCHARGED	
S1.002	S21	1.43			106.8	SURCHARGED	
S1.003	S22	0.63			139.5	SURCHARGED	
S3.000	S23A	0.60			23.1	FLOOD RISK	
S1.004	S23	1.09			193.4	SURCHARGED	
S1.005	S24	2.04			283.6	SURCHARGED	
S1.006	S27	2.75			281.2	SURCHARGED	
S1.007	S28	2.06			281.6	SURCHARGED	
S1.008	S30	2.33			282.0	SURCHARGED	

FOUL SEWERAGE DESIGN

Design Criteria for Foul - Main 1

Pipe Sizes FH-PIPE Manhole Sizes FH-MH

Industrial Flow (1/s/ha) 0.00 Add Flow / Climate Change (%) 0
Industrial Peak Flow Factor 0.00 Minimum Backdrop Height (m) 0.200
Flow Per Person (1/per/day) 222.00 Maximum Backdrop Height (m) 1.500
Persons per House 3.00 Min Design Depth for Optimisation (m) 1.200
Domestic (1/s/ha) 0.00 Min Vel for Auto Design only (m/s) 0.75
Domestic Peak Flow Factor 6.00 Min Slope for Optimisation (1:X) 500

Designed with Level Soffits

Network Design Table for Foul - Main 1

PN	Length	Fall	Slope	Area	Houses	Ba	ase	k	HYD	DIA	Section Type	Auto
	(m)	(m)	(1:X)	(ha)		Flow	(1/s)	(mm)	SECT	(mm)		Design
F1.000	8.696	0.199	43.7	0.000	7		0.0	1.500	0	150	Pipe/Conduit	6
F1.001	14.859	0.256	58.0	0.000	13		0.0	1.500	0	150	Pipe/Conduit	₩.
F1.002	27.882	1.359	20.5	0.000	3		0.0	1.500	0	150	Pipe/Conduit	&
F2.000	31.512	1.185	26.6	0.000	6		0.0	1.500	0	150	Pipe/Conduit	& &
F2.001	27.155	0.235	115.6	0.000	4		0.0	1.500	0	150	Pipe/Conduit	8
F1.003	20.949	0.209	100.2	0.000	8		0.0	1.500	0	150	Pipe/Conduit	&
F1.004	20.931	0.206	101.6	0.000	4		0.0	1.500	0	150	Pipe/Conduit	a A

Network Results Table

PN	US/IL	Σ Area	Σ Base	Σ Hse	Add Flow	P.Dep	P.Vel	Vel	Cap	Flow
	(m)	(ha)	Flow (1/s)		(1/s)	(mm)	(m/s)	(m/s)	(1/s)	(1/s)
F1.000	12.619	0.000	0.0	7	0.0	13	0.45	1.33	23.5	0.3
F1.001	12.420	0.000	0.0	20	0.0	22	0.57	1.15	20.3	0.9
F1.002	12.164	0.000	0.0	23	0.0	18	0.86	1.94	34.3	1.1
F2.000	12.225	0.000	0.0	6	0.0	11	0.51	1.70	30.1	0.3
F2.001	11.040	0.000	0.0	10	0.0	19	0.36	0.81	14.4	0.5
F1.003	10.805	0.000	0.0	41	0.0	36	0.59	0.87	15.5	1.9
F1.004	10.596	0.000	0.0	45	0.0	37	0.60	0.87	15.4	2.1

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Innovyze	Network 2020.1	,

Manhole Schedules for Foul - Main 1

MH Name	MH CL (m)	MH Depth (m)	MH Connection	MH Diam.,L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Backdrop (mm)
F2	14.630	2.011	Open Manhole	1200	F1.000	12.619	150				_
F3	14.495	2.075	Open Manhole	1200	F1.001	12.420	150	F1.000	12.420	150	
F4	14.201	2.037	Open Manhole	1200	F1.002	12.164	150	F1.001	12.164	150	
F7	13.900	1.675	Open Manhole	1200	F2.000	12.225	150				
F8	14.251	3.211	Open Manhole	1200	F2.001	11.040	150	F2.000	11.040	150	
F5	13.765	2.960	Open Manhole	1200	F1.003	10.805	150	F1.002	10.805	150	
								F2.001	10.805	150	
F6	13.406	2.810	Open Manhole	1200	F1.004	10.596	150	F1.003	10.596	150	
FBRF21	13.299	2.909	Open Manhole	0		OUTFALL		F1.004	10.390	150	

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
F2	617059.815	166695.934	617059.815	166695.934	Required	0-
F3	617068.438	166694.808	617068.438	166694.808	Required	
F4	617081.742	166688.458	617081.742	166688.458	Required	
F7	617138.354	166719.357	617138.354	166719.357	Required	,
F8	617123.604	166691.510	617123.604	166691.510	Required	1
F5	617104.443	166672.269	617104.443	166672.269	Required	
F6	617092.567	166655.011	617092.567	166655.011	Required	6
FBRF21	617082.287	166636.779			No Entry	1

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Innovyze	Network 2020.1	1

PIPELINE SCHEDULES for Foul - Main 1

Upstream Manhole

PN	-	Diam (mm)		C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
F1.000	0	150	F2	14.630	12.619	1.861	Open Manhole	1200
F1.001	0	150	F3	14.495	12.420	1.925	Open Manhole	1200
F1.002	0	150	F4	14.201	12.164	1.887	Open Manhole	1200
F2.000	0	150	F7	13.900	12.225	1.525	Open Manhole	1200
F2.001	0	150	F8	14.251	11.040	3.061	Open Manhole	1200
F1.003	0	150	F5	13.765	10.805	2.810	Open Manhole	1200
F1.004	0	150	F6	13.406	10.596	2.660	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)	MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
F1.000	8.696	43.7	F3	14.495	12.420	1.925	Open Manhole	1200
F1.001	14.859	58.0	F4	14.201	12.164	1.887	Open Manhole	1200
F1.002	27.882	20.5	F5	13.765	10.805	2.810	Open Manhole	1200
F2 000	31.512	26 6	F8	14.251	11.040	3 061	Open Manhole	1200
	27.155			13.765			Open Manhole	
							_	
F1.003	20.949	100.2	F6	13.406	10.596	2.660	Open Manhole	1200
F1.004	20.931	101.6	FBRF21	13,299	10.390	2.759	Open Manhole	0



D.	. Southern Water Technical Approval Letter					





Mr Thomas Hawes Fairhurst 13 Park Street London SE1 9EA Your ref

Our ref DSA000000405

Date 10th May 2022

Contact

Tel 0330 303 0119

Dear Mr. Hawes,

Adoption of Sewers – Section 104, Water Industry Act 1991 Development: Herne Bay Phase 4, Land at Herne Bay Golf Course Development, Kent, CT6 7PG.

Thank you for your letter dated 03 May 2022 with enclosures.

I have now completed technical assessment of your submission. Based on an estimated cost of for constructing the proposed sewerage offered for adoption, the fees for design checks, administration, and site supervision amount to in accordance with the scale of charges set out in "Sewers for Adoption – 6th edition". I enclose a remittance advice sheet for that amount less any previous payment and I ask that this be returned with your client's payment to the address given at the bottom of the sheet. Legal fees will be separately invoiced by Southern Water's solicitor and must be paid before the agreement is engrossed. If there is a pumping station to be included within the S.104 Agreement we will request a further fee at a later date, but prior to handover for the signage, padlocks, monitoring and configuration of the telemetry unit.

In addition to the above fees a Bond is required and, based on the estimated cost of construction, the Agreement will incorporate a Bond in the sum of

I am also pleased to confirm that the technical assessment of your proposals is satisfactory and I can now proceed towards drawing up the agreement, which will be based on the model form published in the Sixth edition of "Sewers for Adoption". This document is regarded favorably by the House Builders Federation and Water UK and is the standard against which sewers are adopted by agreement. As a consequence, Southern Water will not agree to departures from that model form and you are strongly advised to make your solicitor aware of this, which will pave the way for a swift production and completion of the agreement.

This letter is for the purpose of technical review of submitted proposals, which have been checked for compliance with Sewers for Adoption and published Technical addendum and/or corrigendum.

Southern Water, Southern House, Yeoman Road, Worthing, West Sussex, BN13 3NX southernwater coluk

It is also an offer to enter into a Section 104 agreement. Please note that this offer letter does not necessarily confirm that capacity is available for the proposed development in the public sewer network. This is not an approval to connect to the public sewer, either directly or indirectly. Such connection will require S106 application and approval. 104 compliance does not necessarily infer that planning conditions can be discharged.

Please note that the Bond must be arranged before any work on site is undertaken and the fees paid not later than the time of the first inspection by our Assistant Project Managers. A minimum of 48 hours notice of your intention to commence the Works should be given to this office. Please be aware that commencing the Works prior to the signing of the Agreement will not absolve you of your responsibility to construct the Works in accordance with the requirements of "Sewers for Adoption" and the relevant drawings.

In order that an Agreement may be drawn up I should be obliged if you would please provide the drawings for the agreement in accordance with the attached specification sheet. Please ensure easements are shown for pipe work over 300mm or 3m deep and for the outfall. N.B. The drawings deemed to comply with "Sewers for Adoption" are numbered:

Drawing No	Revision	Drawing Name
132592- FH-ZZ-00-DR-D- 0501,502,	C7	Main Drainage Layout Sheet 1,2 of 7
132592- FH-ZZ-00-DR-D- 0503,504	C6	Main Drainage Layout Sheet 3,4 of 7
132592- FH-ZZ-00-DR-D- 0209,211,212	C6	Main Drainage Layout Sheet 5,6,7 of 7
132592- FH-ZZ-00-DR-D- 0513	C3	Main Drainage Construction Details
132592- FH-ZZ-00-DR-D- 0520	C1	Headwall Construction Details
132592- FH-ZZ-00-DR-D- 0519	C4	Foul Water Manhole Schedule
132592- FH-ZZ-00-DR-D- 0518	C4	Surface Water Manhole Schedule
132592- FH-ZZ-00-DR-C- 0113,0114,0115	C3	Longitudinal Section Sheet 1,2 & 3

Where applicable, copies of any consents issued by the Environment Agency and/or the Land Drainage Authority to discharge surface water and/or a pumping station emergency overflow into a local watercourse will be required.

I also ask that you complete sections 1 to 11 and 15 of the attached Instruction Sheet, so that Southern Water's solicitor can enter the correct information into the draft agreement. Instructions will not be issued to Southern Water's solicitor until the complete package of this Instruction Sheet and the drawings requested above has been received.

Yours sincerely,

Ashley Marshman

Head of Business Channels

Business Channels

SECTION 104 DEVELOPMENT DETAILS

1 *	Owner's Name	
2 *	Owner's Address	
	(If Company-Registered Office)	
3 *	Developer's Name	
4 *	Developer's Address	
•	(If Company-Registered Office)	
	(iii campany riagionale amony	
5 *	Surety's Name	
<mark>6 *</mark>	Surety's Address	
7 *	Solicitor's Name	
<mark>8 *</mark>	Solicitor's Email Address	
9 *	Solicitor's Address	
10 *	Site Address	
<mark>10 *</mark>	Site Address	
11 *	Site Layout Plan Number(s)	
<mark>12 *</mark>	Other Drawing Numbers	
13	Easements (yes/no)	Yes
14	Pumping Station (yes/no)	No
15	Southern Water Reference No.	DSA00000405
<mark>16 *</mark>	Construction Period	
17	Bond Value (£)	
18	Supervision Fee (£)	

^{*} Indicates information required from Developer

SOUTHERN WATER SERVICES LIMITED REMITTANCE ADVICE SHEET SECTION 104 WATER INDUSTRY ACT 1991

Developer: Mr.Glenn Holliday

Development: Herne Bay Phase 4, Land at Herne Bay Golf Course Development, Kent, CT6 7PG	
Applicant details: Name: Mr Thomas Hawes	Payer details if different: Name:
Address: Fairhurst 13 Park Street London SE1 9EA	Address:
Contact tel:	Contact tel:
Signed	Print name:
Company: Position:	
Date:	
Cheque to the value of £10,769.25 in payment of the following fees:	
325050 A-DEVSERV-S104 Technical checks and supervision Less Deposit previously paid	

DSA000000405

Return to: Developer Services, Southern Water, Southern House, Yeoman Road, Worthing, West Sussex, BN13 3NX.

In order for us to process payment this remittance advice sheet needs to be completed and returned with your payment. Should this not be provided this will cause delays in processing your payment.

SPECIFICATION SHEET FOR DRAWING REQUIREMENTS

WHERE NO EASEMENT IS REQUIRED

2 Copies of the drainage layout drawing to show: -

- Foul water sewers coloured brown with directional arrows (if applicable).
- Surface water sewers coloured blue with directional arrows (if applicable).
- Site boundary edged in green.
- Scale.
- North Point.

2 copies of the Long Section drawing(s).

2 copies of the Typical Detail drawing(s).

2 copies of a Separate Site Location plan (1:1250 extract from Ordnance Survey map with site boundary edged green).

WHERE AN EASEMENT IS REQUIRED

2 Copies of the drainage layout drawing to show: -

- Foul water sewers coloured brown with directional arrows (if applicable).
- Surface water sewers coloured blue with directional arrows (if applicable).
- Site boundary edged in green.
- Scale.
- North Point
- Easements coloured solid yellow with the easement width dimensioned.

2 copies of the Long Section drawing(s).

2 copies of the Typical Detail drawing(s).

2 copies of a Site Location plan (1:1250 extract from Ordnance Survey map with site boundary edged green).

CONVEYANCING PLANS (Where required)

2 Copies of drawings to show: -

- The extent of the pumping station land to be transferred to Southern Water edged in red.
- Any non-adopted highways over which rights of access are to be reserved coloured solid brown.
- These plans must be suitable for land registry purposes i.e. 1:1250 OS based with sufficient background to identify the land and a north point on the drawings. The scale must also be shown.

